

CODES/DESIGN CRITERIA

- A. CODES: - THE EIGHTH EDITION OF THE FLORIDA BUILDING CODE (2023)
- ASCE 7-22
- B. DESIGN LOADS:
DEAD LOADS
PARTITION = 15 PSF
DECK + TOPPING = 43.5 PSF
LIVE LOADS
OFFICE = 50 PSF
STAIRS = 100 PSF
TORNADO LOADS = N/A
- C. ULTIMATE DESIGN WIND SPEED (V_{ULT}): 140 MPH
3 SECOND GUST PER ASCE 7-22, $V_{50} = 108$ MPH
ENCLOSED STRUCTURE (INTERNAL PRESSURE COEFFICIENT = +/- 0.18)
(CATEGORY III) EXPOSURE B
- D. EARTHQUAKE DESIGN DATA:
SEISMIC DESIGN CATEGORY A; ANALYSIS/ LOADS: N/A
 $S_D = 0.07$; $S_M = 0.031$; $S_{DS} = 0.063$; $S_{D1} = 0.044$
SEISMIC IMPORTANCE FACTOR (I_h) = 1.0; SITE CLASS D
- E. STANDARDS:
AISC AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ANSI AMERICAN NATIONAL STANDARDS INSTITUTE
ASTM AMERICAN SOCIETY OF TESTING AND MATERIALS
AWS AMERICAN WELDING SOCIETY
NDS NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION
- F. IN WIND BORNE DEBRIS REGIONS (140 MPH AND GREATER) GLAZED OPENINGS IN BUILDINGS SHALL BE IMPACT RESISTANT OR PROTECTED WITH IMPACT RESISTANT COVERING PER FBC 1609.1.2

GENERAL NOTES

- A. THE GENERAL CONTRACTOR SHALL REVIEW ALL PROJECT DOCUMENTS PRIOR TO FABRICATION AND START OF CONSTRUCTION AND TO REPORT ANY DISCREPANCIES TO THE ARCHITECT/ENGINEER.
- B. THE GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE AND WORKERS DURING CONSTRUCTION, INCLUDING ADEQUATE SHORING AND BRACING OF STRUCTURAL MEMBERS FOR CONSTRUCTION LOADS INCLUDING WIND.
- C. THE GENERAL CONTRACTOR SHALL PROTECT EXISTING FACILITIES, STRUCTURES AND UTILITY LINES FROM ALL DAMAGE DURING CONSTRUCTION.
- D. NO STRUCTURAL MEMBERS SHALL BE CUT, NOTCHED OR REDUCED IN STRENGTH WITHOUT WRITTEN APPROVAL FROM THE ENGINEER.
- E. REPRODUCTION OF THE CONTRACT DOCUMENTS TO BE USED AS SHOP DRAWING SUBMITTALS WILL NOT BE PERMITTED.
- F. THE STRUCTURE IS NOT FULLY STABLE UNTIL ALL CONNECTIONS ARE COMPLETE AND SUPPORT ANCHORAGES MADE.
- G. ALL DIMENSIONS HAVE BEEN BASED ON INFORMATION SUPPLIED. IT IS THE FABRICATOR'S RESPONSIBILITY TO HAVE ALL DIMENSIONS VERIFIED W/ FIELD CONDITIONS PRIOR TO FABRICATION. WILSON & GIRGENTI, LLC IS NOT RESPONSIBLE FOR DIMENSIONING RELATED TO SITE CONDITIONS.
- H. ALL ANCHORS ARE TO BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PROCEDURES AND RECOMMENDATIONS.

STRUCTURAL STEEL

- A. ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATION, UNLESS NOTED OTHERWISE ON PLAN:
WIDE FLANGE ASTM A992
HSS (RECTANGULAR) ASTM A500 GR. C
HSS (ROUND) ASTM A500 GR. C
HSS (PIPES) ASTM A53 GR. B
ROLLED C & MC SHAPES ASTM A36
ANGLES ASTM A36
- B. ALL BOLTS SHALL CONFORM TO ASTM A325U, PROVIDE HARDENED WASHERS UNDER PART BEING TURNED.
- C. ALL CONNECTIONS SHALL CONFORM TO THE MINIMUM STANDARDS (U.N.O.):
1. MINIMUM PLATE THICKNESS: 3/8"
2. MINIMUM BOLT DIAMETER: 3/4"
3. MINIMUM WELD: 3/16"
- D. MINIMUM DESIGN LOAD ON ANY CONNECTION: 16 KIPS
- E. ANCHOR RODS SHALL CONFORM TO ASTM F1554, SEE SCHEDULE FOR GRADE.
- F. ALL SHOP AND FIELD WELDING SHALL CONFORM TO AWS STRUCTURAL WELDING CODE, ANSI/AWS D1.1.
- G. DESIGN CONNECTIONS FOR ALL BEAMS TO SUPPORT HALF OF THE UNIFORM CAPACITY SHOWN IN AISI "TABLES FOR ALLOWABLE LOADS ON BEAMS" FOR THE GIVEN SECTION AND SPAN UNLESS OTHERWISE SHOWN ON PLAN.
- H. GROUT BELOW BASE PLATES SHALL BE NON-SHRINK, NON-METALLIC GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 6000 PSI
- I. HOT-DIP GALVANIZE ALL STRUCTURAL STEEL ITEMS AND THEIR CONNECTIONS PERMANENTLY EXPOSED TO WEATHER AFTER FABRICATION. TOUCH UP ALL IMPERFECTIONS IN FIELD WITH A ZINC BASED PAINT.
- J. FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AISI "MANUAL OF STEEL CONSTRUCTION", 15TH EDITION AND THE AISI "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS."

ADHESIVE AND MECHANICAL FASTENERS

- A. GENERAL
1. ALL ANCHORS ARE TO BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PROCEDURES AND RECOMMENDATIONS.
2. ALL ANCHORS ARE SUBJECT TO TESTING AT THE ENGINEER'S DISCRETION.
- B. ALTERNATE MANUFACTURES MAY BE USED WITH WRITTEN APPROVAL BY THE ENGINEER. ALL SUGGESTED ALTERNATES ARE TO HAVE ICBO APPROVAL.

METAL DECK

- A. GENERAL
1. THE METAL DECK AND INSTALLATION ARE TO COMPLY WITH THE REQUIREMENTS OF THE STEEL DECK INSTITUTE (SDI).
- a. REFERENCE CONSTRUCTION DOCUMENTS FOR TYPE, SIZE, GAGE AND FINISH OF METAL DECK.
- b. DECK SHALL BE PLACED IN A MINIMUM THREE SPAN CONDITION. SINGLE SPANS ARE NOT PERMITTED UNLESS SPECIFICALLY SHOWN IN THE DOCUMENTS.
- c. PROVIDE A MINIMUM BEARING OF 2" AT ALL INTERIOR SUPPORTS.
- d. PROVIDE MINIMUM BEARING LENGTH OF 2" ALL EXTERIOR SUPPORTS.
- e. PROVIDE CONTINUOUS 18 GAGE RIDGE PLATES AND VALLEY PLATES AS REQUIRED.
- f. WELDING OF DECK SHALL BE IN ACCORDANCE WITH ANSI/AWS D1.3 "STRUCTURAL WELDING CODE - SHEET STEEL"
- g. STEEL DECK SHALL BE STORED OFF THE GROUND WITH ONE END ELEVATED TO PROVIDE DRAINAGE.
- h. DO NOT ATTACH ANY MECHANICAL ITEMS SUCH AS LIGHT FIXTURES, DUCTS, AND PIPING TO THE METAL DECK UNLESS APPROVED BY THE ENGINEER.

CONTRACTOR RESPONSIBILITY

- A. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND, EXCEPT WHERE SPECIFICALLY SHOWN, DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, AND SEQUENCE. THE ENGINEER SHALL NOT HAVE CONTROL OR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, OR SEQUENCE, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING AND OF THE WORK, OR FOR THE FAILURE IF AND OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

FOUNDATION

- A. GEOTECHNICAL REPORT
1. FOUNDATION DESIGN IS BASED ON A SOIL BEARING PRESSURE OF 1500 PSF.
2. THE GENERAL CONTRACTOR AND EARTHWORK SUBCONTRACTORS SHALL CONFIRM THIS VALUE BY TESTING. THE SITE SHALL BE PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE REPORT.
3. THE UPPER 12" OF BEARING SOIL IN FOOTING SHALL BE COMPACTED TO 95% OF THE STANDARD PROCTOR.
4. REMOVE TOPSOIL & ORGANIC MATERIAL FROM TOP 12" OF EXISTING GRADE.
5. PROOF ROLL OF 5' OUTSIDE BUILDING FOOTPRINT WITH VIBRATORY COMPACTOR.
6. FILL TO WITHIN 4" OF FINISHED FLOOR ELEVATION WITH CLEAN SAND FILL.
7. COMPACT TOP 6" OF FILL MATERIAL TO 95% OF MODIFIED PROCTOR DENSITY. (MIN.)
8. SOIL IN FOOTING TRENCHES SHALL BE FREE OF ORGANIC MATERIAL OR CLAY: IF EITHER IS ENCOUNTERED IN FOOTING TRENCHES, REMOVE IT & REPLACE WITH COMPACTED SAND.
- B. CONCRETE PLACEMENT
1. CONCRETE PLACEMENT SHALL OCCUR IMMEDIATELY AFTER FOOTING EXCAVATION AND PLACEMENT OF REINFORCEMENT.
2. ANY FREE STANDING WATER SHALL BE PUMPED OUT OF THE EXCAVATION PRIOR TO CONCRETE PLACEMENT.

CONCRETE AND REINFORCING STEEL

LOCATION	28-DAY COMP. STRENGTH (PSI)	MAX SIZE AGGREGATE	SLUMP RANGE	MAX. W/C RATIO
FOUNDATIONS	3000 NW	3"	9" TO 11"	0.55
SLABS	3000 NW	1"	3" TO 6"	

NW = NORMAL WEIGHT CONCRETE (145 PCF)

- A. ALL REINFORCING STEEL TO BE NEW BILLET STEEL, CONFORMING TO ASTM A-615, GRADE 60.
- B. ALL WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A-185.
- C. REINFORCE HOUSEKEEPING AND TOPPING SLABS WITH WWR 6X6-W2.9XW2.9 UNLESS HEAVIER REINFORCEMENT IS CALLED FOR ON THE DRAWINGS.
- D. PROVIDE COVER TO ALL REINFORCEMENT MEETING THE REQUIREMENTS OF ACI 318.
- E. SLAB-ON-GRADE CONCRETE MIX DESIGN TO INCLUDE HIGH-RANGE WATER REDUCING ADMIXTURE.
- F. ALL REBAR SHALL HAVE A LAP LENGTH OF 48 BAR DIAMETERS MINIMUM UNLESS NOTED OTHERWISE.
- G. AT CORNERS OF FOUNDATIONS AND WALLS, "L" SHAPED CORNER BARS SHALL BE PROVIDED TO MATCH THE BAR SIZE INDICATED FOR THE FOUNDATION OR WALL. BARS SHALL LAP WITH STRAIGHT BARS WHICH RUN PARALLEL TO THE MEMBER IN EITHER DIRECTION.

CONCRETE MASONRY

- A. GENERAL
1. ALL WALL LOCATIONS TO BE COORDINATED BY CONTRACTOR.
- B. CONCRETE MASONRY UNITS
1. CONCRETE STRENGTH OF MASONRY UNITS (BASED ON NET AREA) SHALL BE AT LEAST 1900 PSI (F_m=1500PSI). UNITS SHALL BE NORMAL WEIGHT UNLESS NOTED OTHERWISE.
2. UNITS SHALL CONFORM TO ASTM C55 OR ASTM C90 AND SAMPLED IN ACCORDANCE WITH ASTM C140.
- C. MORTAR
1. TYPICAL WALL - USE MASONRY CEMENT, TYPE M OR S, MORTAR CONFORMING TO ASTM C270. PROVIDE AN AVERAGE COMPRESSIVE STRENGTH AT 28 DAYS OF AT LEAST 1800 PSI.
2. RETAINING WALLS AND MASONRY BELOW GRADE - USE MASONRY CEMENT, TYPE M, MORTAR CONFORMING TO ASTM C270. PROVIDE AN AVERAGE COMPRESSIVE STRENGTH AT 28 DAYS OF AT LEAST 2500 PSI.
- D. GROUT
1. MIX DESIGNS
- a. FOR FILLING SPACES 4" OR LARGER IN BOTH HORIZONTAL DIRECTIONS, USE "COARSE GROUT" WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.
- b. USE 3000 PSI NORMAL-WEIGHT CONCRETE FOR FILLING SPACES 10" AND LARGER IN BOTH DIRECTIONS.
- c. ALL GROUT MIX DESIGN SUBMITTALS SHALL INCLUDE THE RESULTS OF THE TESTS PERFORMED IN ACCORDANCE WITH ASTM C1019.
2. LIFT LIMITS
- a. GROUT POURS SHALL NOT EXCEED 5 FT/LIFT, UNLESS CLEANOUTS ARE PROVIDED IN THE BOTTOM COURSE OF EACH 5 FOOT LIFT.
- b. MECHANICALLY VIBRATE ALL LIFTS IN EXCESS OF 1 FOOT.
- c. SHALL BE STOPPED 1 1/2" BELOW BED JOINT.
3. PROVIDE CLEAN-OUTS FOR ALL GROUT POURS EXCEEDING 5 FEET.
- E. REINFORCEMENT
1. PROVIDE VERTICAL REINFORCEMENT IN CELLS OF CONCRETE MASONRY UNITS (FULLY EMBEDDED IN GROUT) AS SHOWN ON DRAWINGS. MINIMUM REINFORCEMENT OF EXTERIOR, PERIMETER, OR LOAD-BEARING MASONRY SHALL BE AS FOLLOWS:
- a. (3) REINFORCED CELLS AT EACH CORNER
- b. (1) REINFORCED CELL AT ALL INTERSECTIONS AND CONTROL JOINTS.
- c. (2) REINFORCED CELLS EACH SIDE OF OPENINGS UP TO 6 FEET WIDE
- d. SPACE REINFORCEMENT AT 24" ON CENTER OTHERWISE.
2. AT A MINIMUM, PROVIDE HORIZONTAL BOND BEAMS REINFORCED WITH (3) #6 AT EVERY FLOOR LEVEL AND AT THE ROOF. - SEE SECTIONS & DETAILS.
3. TERMINATE ALL VERTICAL REINFORCEMENT IN THE BOND BEAM AT ROOF LEVEL WITH A STANDARD HOOK. TERMINATE AT HIGHEST BOND BEAM IF MASONRY DOES NOT EXTEND TO ROOF OR PLOUCEMENT CELL IS NOT CONTINUOUS TO ROOF. THE HOOK SHALL EXTEND TO THE UPPERMOST HORIZONTAL REINFORCEMENT AT THE BOND BEAM AND SHALL HAVE A MINIMUM EMBEDMENT AT 6".
4. TERMINATE ALL HORIZONTAL REINFORCEMENT OF DISCONTINUOUS ENDS OF BOND BEAM AND INTO VERTICAL GROUTED CELLS WITH A STANDARD HOOK. PROVIDE CORNER BARS SUCH THAT HORIZONTAL REINFORCEMENT IS CONTINUOUS AROUND BUILDING CORNERS.
5. MINIMUM LAP OF ALL REINFORCEMENT SHALL BE PER THE FLORIDA BUILDING CODE, 2020. DO NOT LAP VERTICAL REINFORCEMENT AT INTERSECTING BOND BEAMS; REINFORCEMENT SHALL BE CONTINUOUS THROUGH INTERSECTING BOND BEAMS.
6. PROVIDE HORIZONTAL REINFORCEMENT IN BED JOINTS EVERY OTHER COURSE (MAX. 16" SPACING). REINFORCEMENT SHALL BE LADDER-TYPE WITH 9 GAGE SIDE RAILS FABRICATED FROM HIGH-STRENGTH, COLD-DRAWN WIRE CONFORMING TO ASTM A82. TRUSSES SHALL BE GALVANIZED AFTER FABRICATION. ALSO PLACE THREE ROWS OF REINFORCEMENT AT 8" O.C. IMMEDIATELY ABOVE ALL WALL OPENINGS, AND AT THE TOPS OF WALLS. SIDE LAP RAILS A MINIMUM OF 6". FILL SOLID & REINFORCEMENT FROM WINDOWS & DOOR HEADS TO BOND BEAM ABOVE.

- F. NOTES
1. MASONRY WALLS SHALL BE BRACED EITHER BY OTHER INTERSECTING WALLS OR BY ANCHORS TO THE STRUCTURE ABOVE, AS SHOWN IN THE STRUCTURAL DRAWINGS.
2. BLOCK LINTELS SHALL BE SPECIALLY FORMED U-SHAPED LINTEL UNITS WITH REINFORCING BARS, OR PRECAST UNITS DESIGNED FOR THE WEIGHT OF MASONRY ABOVE. SEE TYPICAL DETAILS. TEMPORARILY SHORE ALL LINTELS AS REQUIRED TO PREVENT DEFLECTION THAT MAY OCCUR BEFORE GROUT HAS CURED.
3. ALL MASONRY WALLS SHOWN ON THE ARCHITECTURAL AND STRUCTURAL DRAWINGS HAVE BEEN DESIGNED TO RESIST THE REQUIRED CODE VERTICAL AND LATERAL FORCES IN THE FINAL CONSTRUCTED CONFIGURATION ONLY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ADEQUATELY BRACE THE WALLS FOR VERTICAL AND LATERAL LOADS THAT COULD POSSIBLY BE APPLIED PRIOR TO COMPLETION OF CONSTRUCTION.
4. PROVIDE SOLID GROUTED U-BLOCKS UNDER ALL WINDOW SILLS.

WIND CALCULATIONS

HEIGHT - 15'

VELOCITY PRESSURE

$q_z = 0.00256K_zK_{zt}K_dK_eK_gV^2$
 $q_z = 0.00256(0.57)(1.0)(0.85)(1.0)(1/146)^2$
 $q_z = 24.3$ PSF

WIND PRESSURE (ULTIMATE)

$F/A = q_zGC_p$
 $F/A = (24.3)(1.9) = 46.2$ PSF (LATERAL)
 $F/A = (24.3)(1.5) = 36.5$ PSF (VERTICAL)

WIND PRESSURE (WORKING)

$F/A = (0.6)q_z$
 $F/A = (0.6)(46.2) = 27.7$ PSF (LATERAL)
 $F/A = (0.6)(36.5) = 21.9$ PSF (VERTICAL)

LIGHT GAUGE METAL (COLD FORMED STEEL)

- A. GENERAL
1. SEE THE ARCHITECTURAL DRAWINGS FOR THE LOCATIONS AND EXTENT OF ALL STEEL STUDS. THE SIZES SHOWN IN THE DRAWINGS ARE THE MINIMUM SIZES REQUIRED.
2. THE DESIGN, DETAILING, AND ENGINEERING OF ALL STEEL STUDS IS THE RESPONSIBILITY OF THE STUD MANUFACTURER AND FABRICATOR.
3. SEE THE ARCHITECTURAL SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS NOT INCLUDED IN THIS SECTION.
- B. CODES AND SPECIFICATIONS:
1. MATERIAL: 16 GAGE AND HEAVIER MATERIAL SHALL BE OF A STRUCTURAL QUALITY STEEL SHEET WITH A MINIMUM YIELD POINT OF 50,000 PSI AND CONFORM TO ASTM A446, A570, OR A611.
2. DESIGN:
- a. STEEL STUDS AND THEIR CONNECTIONS SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE "COLD-FORMED STEEL DESIGN MANUAL" PUBLISHED BY THE AMERICAN IRON AND STEEL INSTITUTE (AISI).
- b. COLD-FORMED STEEL MEMBERS SHALL BE DESIGNED TO RESIST ALL WIND LOADS, LIVE LOADS, AND DEAD LOADS. THE LOADS ARE NOTED IN THE SECTION ENTITLED "DESIGN LOADS". IN ADDITION, MEMBERS AND CONNECTIONS SHALL BE DESIGNED FOR ANY REACTIONS FROM CONNECTED COMPONENTS (WINDOWS, DOORS, AND CURTAINWALL, ETC.) AS WELL AS MISCELLANEOUS ITEMS SHOWN ON THE ARCHITECTURAL DRAWINGS. INTERIOR WALLS SHALL BE DESIGNED FOR A MINIMUM LATERAL PRESSURE OF 10 PSF.
- c. CERTAIN MEMBERS AND CONNECTIONS MAY REQUIRE THE WORK OF A SPECIALTY ENGINEER FOR THE DESIGN OF THOSE COMPONENTS.
- 1) CHARTS AND TABLES PUBLISHED BY THE STUD MANUFACTURER MAY BE USED FOR TYPICAL SPANS AND CONNECTIONS. NO SUBMITTALS TO THE ENGINEER OF RECORD ARE REQUIRED FOR TYPICAL INSTALLATIONS, UNLESS NOTED OTHERWISE.
- 2) A SPECIALTY ENGINEER SHALL DESIGN ALL NON-TYPICAL CONDITIONS. SIGNED AND SEALED CALCULATIONS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR RECORD PURPOSES ONLY.
- 3) "NON-TYPICAL" CONDITIONS ARE DEFINED AS THOSE CONDITIONS THAT ARE NOT SPECIFICALLY INCLUDED IN THE MANUFACTURER'S PUBLISHED CHARTS AND TABLES.
- 4) THE FABRICATOR SHALL REVIEW THE CONTRACT DOCUMENTS AND IDENTIFY ALL NON-TYPICAL CONDITIONS.

SUBMITTALS

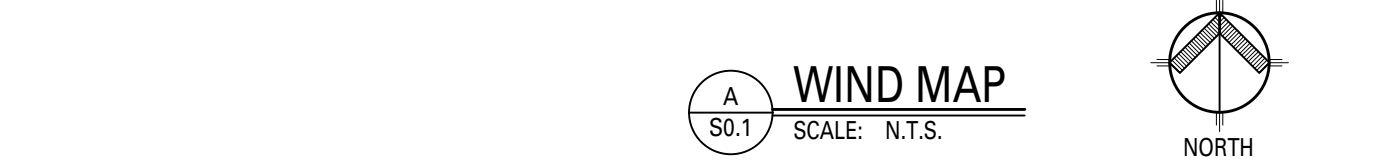
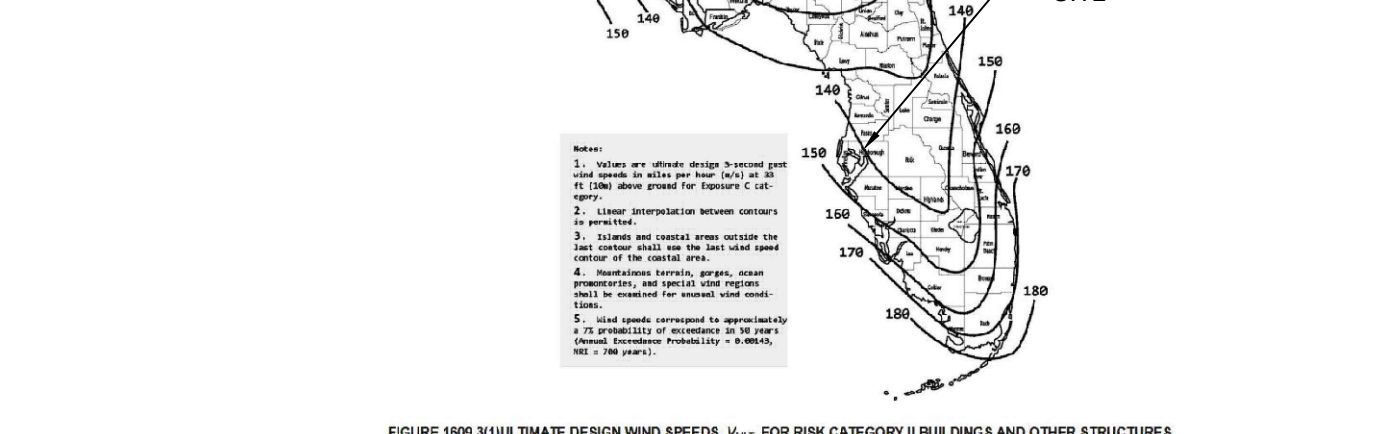
- A. THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS:
1. REINFORCING STEEL
2. CONCRETE MIX DESIGNS
3. STRUCTURAL STEEL(*)
4. OPEN WEB STEEL JOISTS(*) & PRE-FAB STEEL AT SHARED JOISTS.
5. LIGHT GAUGE (COLD FORMED STEEL) METAL FRAMING SUPPORTING EXTERIOR CLADDING SYSTEMS(*)
6. PRE-ENGINEERED CANOPIES (*)
- ITEMS MARKED (*) SHALL HAVE SHOP DRAWINGS SIGNED, SEALED & DATED BY A REGISTERED ENGINEER IN THE STATE OF FLORIDA.
- B. THE GENERAL CONTRACTOR SHALL SUBMIT CALCULATIONS SIGNED, SEALED & DATED BY A REGISTERED ENGINEER IN THE STATE OF FLORIDA FOR THE FOLLOWING ITEMS FOR REVIEW:
1. STRUCTURAL STEEL CONNECTIONS (IF OTHER THAN AS SHOWN ON THE DRAWINGS)
2. FORMWORK, SHORING, BACKSHORING
- C. ALL SHOP DRAWINGS MUST BE REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR AND ARCHITECT PRIOR TO SUBMITTAL FOR FINAL REVIEW BY THE EOR.
- D. SUBMIT MANUFACTURERS LITERATURE FOR ALL MATERIALS AND PRODUCTS USED IN CONSTRUCTION ON THE PROJECT.
- E. COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF EACH BUILDING COMPONENT NOT DESIGNED BY THE DESIGN TEAM OF RECORD AND NOT SPECIFIED ON THE PROJECT CONSTRUCTION DOCUMENTS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT AND SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.
- F. SOME STRUCTURAL SYSTEMS ARE DEFINED AS VENDOR-DESIGNED COMPONENTS PER THE STRUCTURAL DOCUMENTS. THESE ELEMENTS OF THE DESIGN ARE DEFERRED SUBMITTAL COMPONENTS AND HAVE NOT BEEN PERMITTED UNDER THE BASE BUILDING APPLICATION. VENDOR-DESIGNED COMPONENT SHOP DRAWINGS SHALL BE APPROVED BY THE COMPONENT DESIGNER ENGINEER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASE STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. SHOP DRAWINGS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE. THE CONTRACTOR SHALL SUBMIT THE STAMPED COMPONENT SYSTEM DOCUMENTS TO THE BUILDING OFFICIAL FOR APPROVAL.

- WORK ON EXISTING SPACES
- A. THE CONTRACTOR SHALL VERIFY ALL NEW LAYOUTS, DIMENSIONS AND EXISTING CONDITIONS AS SHOWN PRIOR TO CONSTRUCTION AND FABRICATION OF NEW MATERIALS. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE EOR OF ANY HIDDEN OR EXISTING CONDITIONS THAT ARE IN CONFLICT WITH THE PLANS AND DETAILS.
- B. AT NEW CONSTRUCTION ADJACENT TO EXISTING STRUCTURES, THE CONTRACTOR SHALL VERIFY THE LOCATIONS OF NEW FOUNDATIONS RELATIVE TO ALL EXISTING FOUNDATION ELEMENTS. DO NOT UNDERMINE EXISTING FOUNDATIONS (UNLESS SPECIFICALLY DETAILED ON STRUCTURAL DRAWINGS).
- C. DIMENSIONS SHOWN ARE APPROXIMATE AND TO BE USED FOR TAKEOFF PURPOSES ONLY. ALL SHOP DRAWING SUBMITTALS AND FINAL SIZES ARE TO BE BASED UPON FIELD VERIFIED DIMENSIONS.

ABBREVIATIONS AND SYMBOLS

- LLH = LONG LEG HORIZONTAL
- H.S. = HEADED STUD
DBA = DEFORMED BAR ANCHOR
C.J. = CONSTRUCTION OR CONTROL JOINT
CMU = CONCRETE MASONRY UNIT
HSS = HOLLOW STRUCTURAL SECTION
CFMF = COLD FORMED METAL FRAMING
U.N.O. = UNLESS NOTED OTHERWISE
SJI = STEEL JOIST INSTITUTE
GALV. = GALVANIZED
WWR = WELDED WIRE REINFORCEMENT.
REINF. = REINFORCEMENT
GA. = GAUGE
- LSV = LONG SIDE VERTICAL
LSH = LONG SIDE HORIZONTAL
E.F. = EACH FACE
E.W. = EACH WAY
E.S. = EITHER SIDE
T&B = TOP AND BOTTOM
O.C. = ON CENTER
EXP. = EXPANSION
E.J. = EXPANSION JOINT
V.C.J. = VERTICAL CONTROL JOINT
CONT. = CONTINUOUS
- ARCH'L = ARCHITECTURAL
MEP = MECHANICAL, ELECTRICAL & PLUMBING
REF. = REFERENCE
PL = PLATE
STIFF. = STIFFENER
CL = CENTERLINE
VERT. = VERTICAL
HORZ. = HORIZONTAL
REQ'D = REQUIRED
ADD. = ADDITIONAL
LLV = LONG LEG VERTICAL

- ARCH'L = ARCHITECTURAL
MEP = MECHANICAL, ELECTRICAL & PLUMBING
REF. = REFERENCE
PL = PLATE
STIFF. = STIFFENER
CL = CENTERLINE
VERT. = VERTICAL
HORZ. = HORIZONTAL
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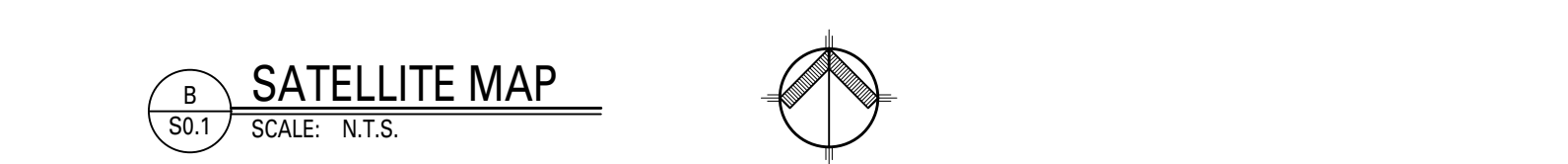
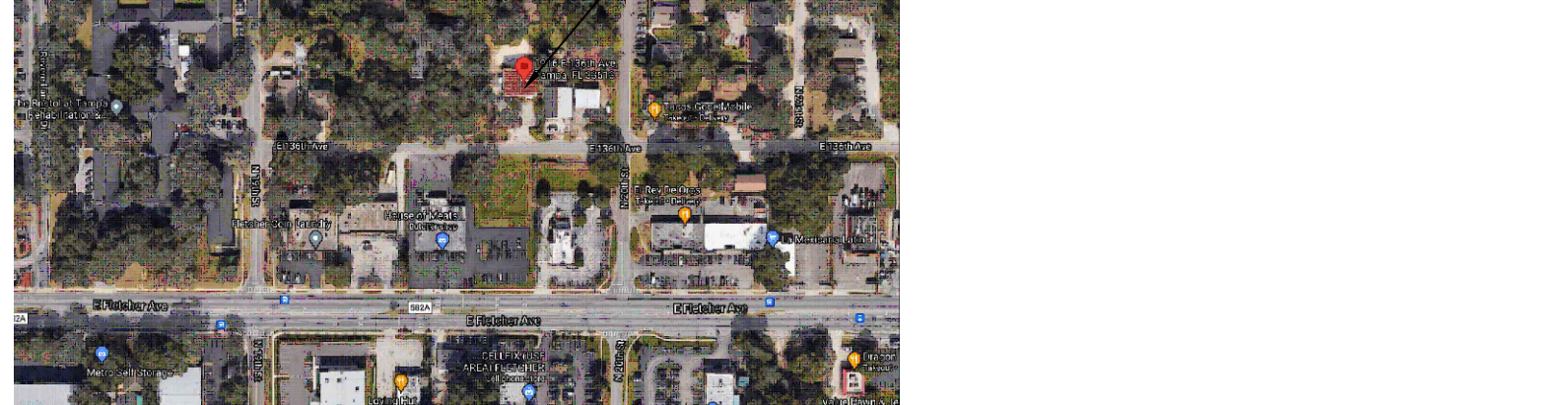
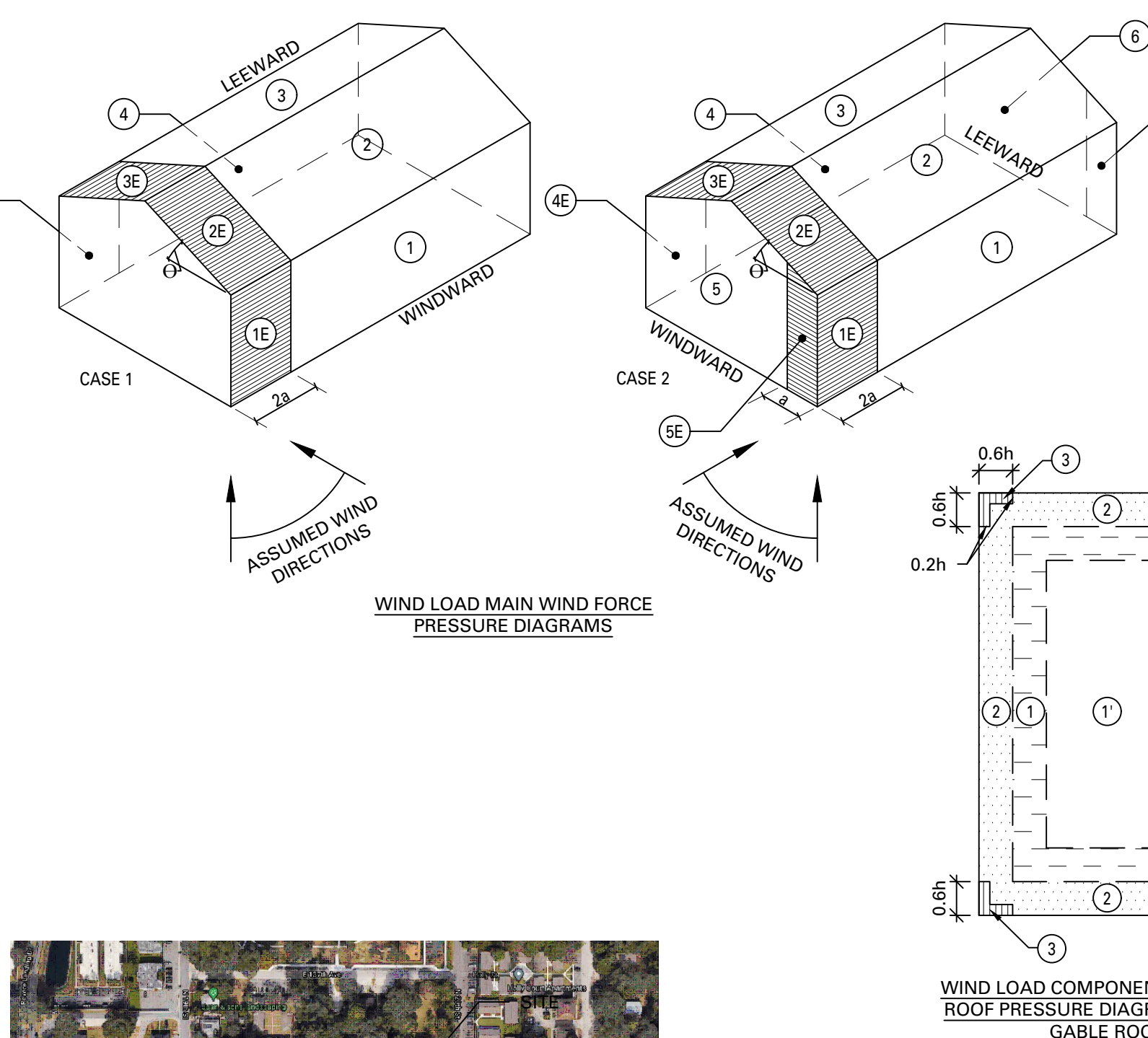


BUILDING DATA		ASCE 7-22 WIND					
WIND VELOCITY V_{ULT}	140 MPH	POSITIVE INTERNAL PRESSURE COEFFICIENT (GC_{pi}):			0.18		
WIND VELOCITY V_{50}	108 MPH	NEGATIVE INTERNAL PRESSURE COEFFICIENT (GC_{pi}):			-0.18		
BUILDING CATEGORY (T 1.5-1)	B	GUST EFFECT FACTOR (26.11.1)			0.85		
WIND EXPOSURE CATEGORY (26.7.3)	ENCLOSED	WIND DIRECTIONALITY FACTOR (K_d) (T 26.6-1)			0.85		
ENCLOSURE	25 FT	VELOCITY PRESSURE EXPOSURE COEFFICIENTS (K_z/K_{zt}) (T 26.10-1)			0.86		
NEAR ROOF HEIGHT		TOPOGRAPHIC FACTOR (K_{zt}) (26.8.2)			1.00		
		GROUND ELEVATION FACTOR (K_{eg}) (T 26.9-1)			1.00		
ROOF TYPE		FLAT ROOF					
ROOF ANGLE (C-I-C)		0-37°					
ROOF ANGLE (MWFRS)		0°-5°					
DESIGN WIND LOADS - WINDOWS, DOORS, COMPONENTS AND CLADDING							
$q_z = 0.00256(K_z)(K_{zt})(K_d)(K_{eg})(V^2)$							
$q_z = 28.15$ PSF							
ROOF					ROOF OVERHANG		
ZONE	AREA (ft²)	$GC_{pi} +$	$GC_{pi} -$	DESIGN PRESSURE (psf)			
				Positive	Negative	Working Uplift	
1'	10	0.30	-0.90	13.5	-30.4	-18.2	
	20	0.27	-0.90	12.7	-30.4	-18.2	
	50	0.23	-0.90	11.5	-30.4	-18.2	
	100	0.20	-0.90	10.7	-30.4	-18.2	
	200	0.20	-0.75	10.7	-28.2	-15.7	
1	10	0.30	-1.70	13.5	-52.9	-31.8	
	20	0.27	-1.58	12.7	-48.4	-29.7	
	50	0.23	-1.41	11.5	-44.8	-26.9	
	100	0.20	-1.29	10.7	-41.3	-24.8	
	200	0.20	-1.16	10.7	-37.8	-22.7	
2	10	0.30	-2.30	13.5	-69.8	-41.9	
	20	0.27	-2.14	12.7	-65.3	-39.2	
	50	0.23	-1.93	11.5	-59.4	-35.6	
	100	0.20	-1.77	10.7	-54.9	-32.9	
	200	0.20	-1.61	10.7	-50.4	-30.2	
3	10	0.30	-3.20	13.5	-95.1	-57.1	
	20	0.27	-2.88	12.7	-86.2	-51.7	
	50	0.23	-2.46	11.5	-74.3	-44.6	
	100	0.20	-2.14	10.7	-65.3	-39.2	
	200	0.20	-1.92	10.7	-59.3	-33.8	
WALLS							
ZONE	AREA (ft²)	$GC_{pi} +$	$GC_{pi} -$	DESIGN PRESSURE (psf)			
				Positive	Negative	Working†	
4	10	1.00	-1.10	33.2	-30.0	-21.6	
	20	0.96	-1.06	32.0	-34.8	-20.9	
	50	0.89	-0.99	30.2	-33.0	-19.8	
	100	0.84	-0.94	28.7	-31.6	-18.9	
	200	0.79	-0.89	27.3	-30.1	-18.1	
5	10	1.00	-1.40	33.2	-44.6	-26.7	
	20	0.96	-1.29	32.0	-41.5	-24.9	
	50	0.89	-1.15	30.2	-37.5	-22.5	
	100	0.84	-1.05	28.7	-34.5	-20.7	
	200	0.79	-0.94	27.3	-31.5	-18.9	

Notes:

- For effective areas between those given above the load may be interpolated, otherwise use the load associated with the lower effective area.
- Plus and minus signs signify pressures acting toward and away from the surfaces, respectively.
- Pressures shown are applied normal to the surface.
- Refer to pressure zone diagrams provided for corresponding zones.
- Roof coverings, finishes, etc. shall be designed for the full negative design pressure.
- Design pressures shown shall be multiplied by the appropriate load use factor from article 2.3.1 or 2.4.1 of ASCE 7-22 when performing strength or stress design on structural elements of building.

DESIGN WIND LOADS - MWFRS		ZONES											
BASIC WIND SPEED (mph)	ROOF ANGLE (DEGREES)	L CASE	WALL PRESSURE FACTORS (GC_{pi})				ROOF PRESSURE FACTORS (GC_{pi})				ROOF PRESSURE (PSF)		
			1	1E	4	5	6	6E	8	8E	2	3	3E
140	0°-5°	1	0.40	0.61	-0.29	-0.43	N/A	N/A	N/A	N/A	-0.69	-1.07	-0.53
	0°-30°	2	-0.45	-0.48	-0.45	-0.48	0.40	0.61	-0.29	-0.43	-0.69	-0.53	-0.37
	0°-5°	1	16.3	22.2	-13.2	-17.2	N/A	N/A	N/A	N/A	-24.5	-35.2	-15.5
	0°-30°	2	-17.7	-18.6	-17.7	-18.6	16.3	22.2	-13.2	-17.2	-24.5	-20.0	-15.5



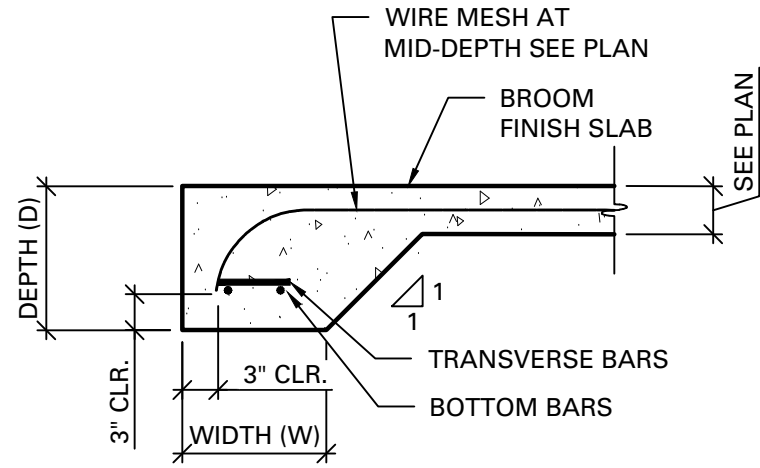
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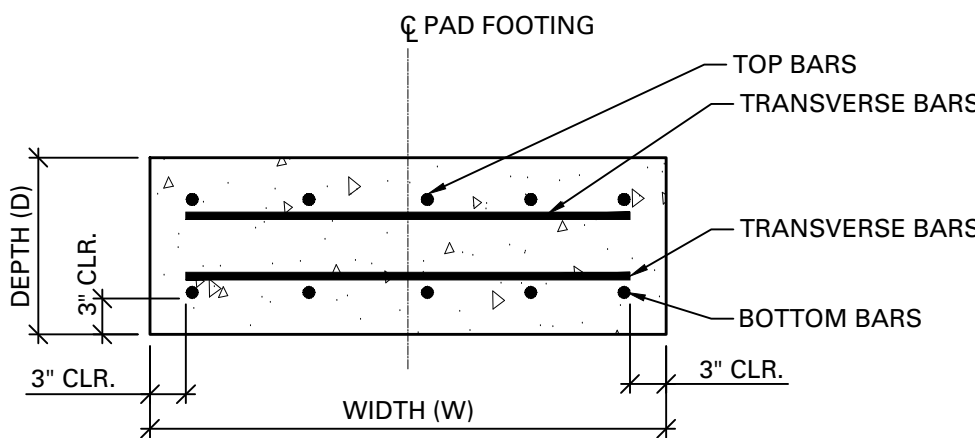
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FOOTING SCHEDULE			
MARK	SIZE WXLXD	REINFORCEMENT	TOP ELEV (U.N.O.)
TD1	1'-0" W X CONT. X 12" D	(2) - #5 BTM. & #4 TRANSVERSE AT 24" O.C.	-
PF	SEE PLAN x 12" D.	#5 AT 10" O.C. EACH WAY BOTTOM (2) #5 MIN.	-

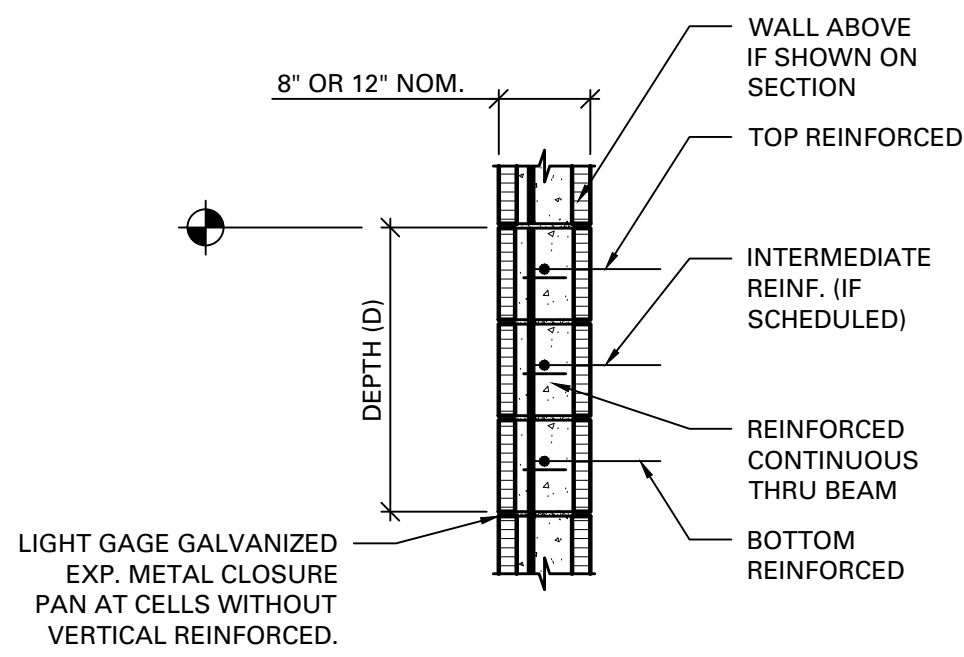
FOOTING
SCHEDULE
SCALE: N.T.S.



TD FOOTING (EDGE)



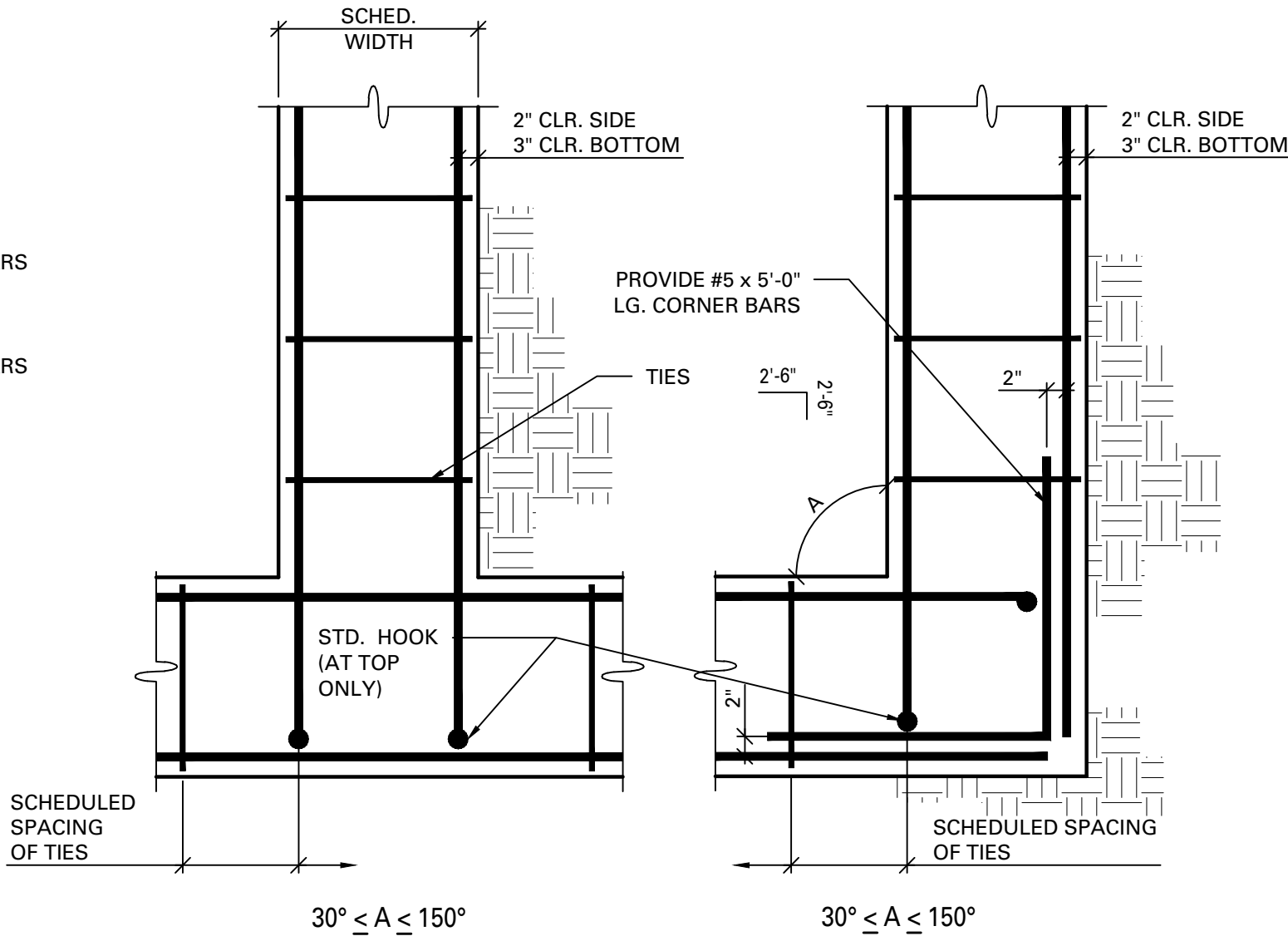
PF FOOTING



BOND BEAM (BB) SCHEDULE						
MARK	SIZE (NOM)		REINFORCEMENT			REMARKS
	W	D	TOP	BOTTOM	INTER-MEDIATE	
8BB1	8"	8"	1 - #5	----	----	
8BB2	8"	16"	1 - #5	1 - #5	----	
8BB3	8"	24"	1 - #5	1 - #5	1 - #5	
8BB4	8"	32"	1 - #5	1 - #5	1 - #5	

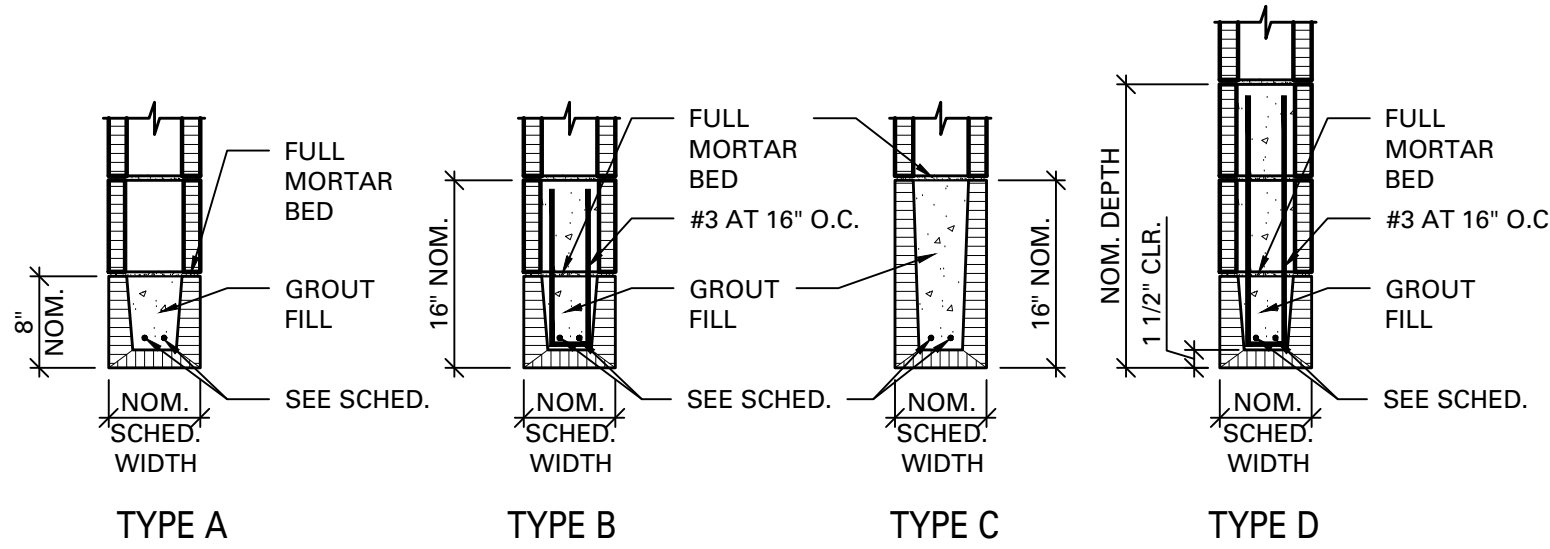
1. PROVIDE 8" PRECAST FOR BOTTOM COURSE WHEN SHOWN AS A LINTEL.

BOND BEAM
SCHEDULE
SCALE: N.T.S.



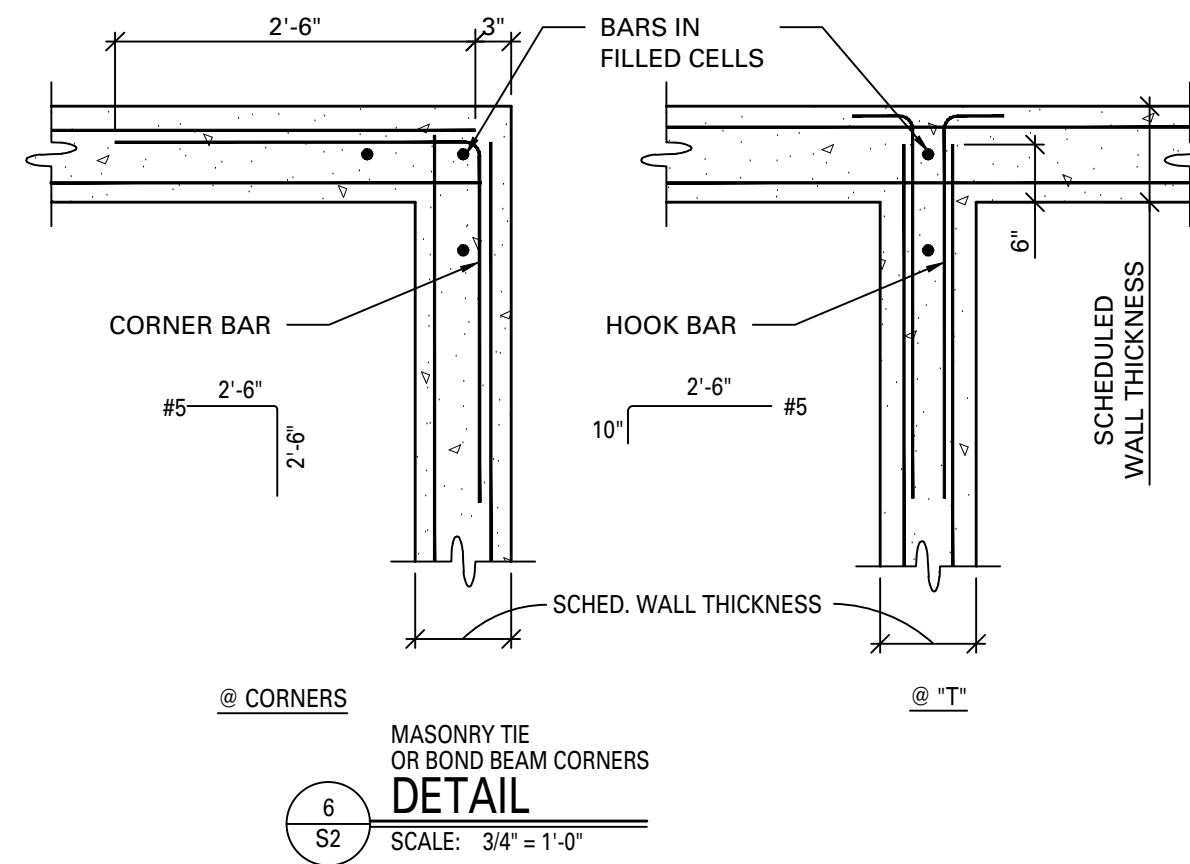
CONT. FOOTING BAR PLACEMENT
DETAIL
SCALE: 3/4" = 1'-0"

LINTEL SCHEDULE				
MARK	SIZE (L x W x D)	REINFORCEMENT	REMARK	TYPE
(L1)	8"x8" NOM.	(1) - #5 REBAR		A
(L5)	8"x16" NOM.	(2) - #5 REBAR	SEE NOTES: 9	B

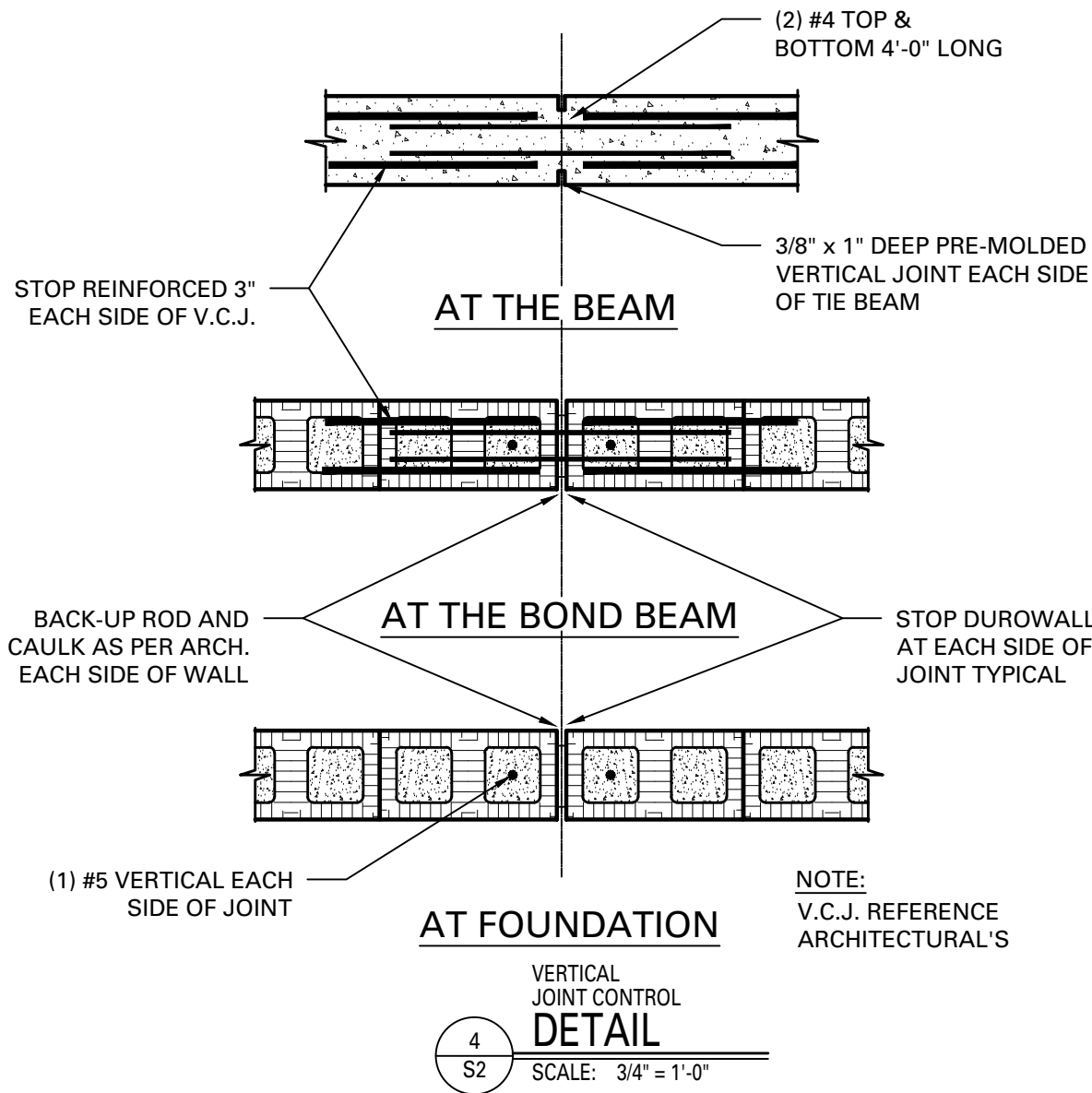


- NOTES:
- SEE ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF OPENINGS.
 - PROVIDE 1" OF BEARING AT EACH JAMB FOR EACH FOOT OF CLEAR SPAN BUT NOT LESS THAN 7 5/8". REINFORCEMENT SHALL PROJECT A MINIMUM OF 6" ONTO THE BEARING.
 - MINIMUM MASONRY COMPRESSIVE STRENGTH OF GROUTED PRISM $f_{m'} \geq 1500$ PSI.
 - LINTELS SCHEDULED AS 16" HIGH MAY BE EITHER TYPE "B" OR TYPE "C" AS DETAILED BELOW.
 - PRECAST CONCRETE LINTELS AS SHOWN MAY BE USED WITH ARCHITECT'S APPROVAL. MIN $f'_c = 3000$ PSI.
 - MINIMUM LINTEL LOAD PER LINEAR FOOT = 1000 LBS.
 - FILL ZONE ABOVE LINTEL UP TO BOTTOM OF OVERHEAD FLOOR BOND BEAM.
 - PROVIDE SHORING FOR LINTEL UNTIL CONCRETE GROUT REACHES .75 f'_c .
 - PROVIDE (2) #5 EACH SIDE OF OPENINGS GREATER THAN OR EQUAL TO 6'-0", AND LESS THAN 12'-0".
 - PROVIDE (3) #5 EACH SIDE OF OPENINGS GREATER THAN 12'-0".

LINTEL
SCHEDULE
SCALE: N.T.S.



MASONRY TIE
OR BOND BEAM CORNERS
DETAIL
SCALE: 3/4" = 1'-0"



VERTICAL
JOINT CONTROL
DETAIL
SCALE: 3/4" = 1'-0"

THE WAREHOUSE
A RENOVATION AT:
1916 E 136 AVE
TAMPA, FL 33613
FOR:
UNIVERSITY AREA COMMUNITY
DEVELOPMENT CORP (UACDC)

NO.	ISSUE DESCRIPTION	DATE
05	100% CD SET	02-21-24
06	HILLS CTY PPR	06-13-24
07	PERMIT SET	09-25-24
09	BIDDING DOCUMENTS	04-19-25

PROFESSIONAL SEAL

DRAWN: DS
REVIEWED: TL
PROJECT NO: 1805

SCHEDULES AND
DETAILS

S0.2



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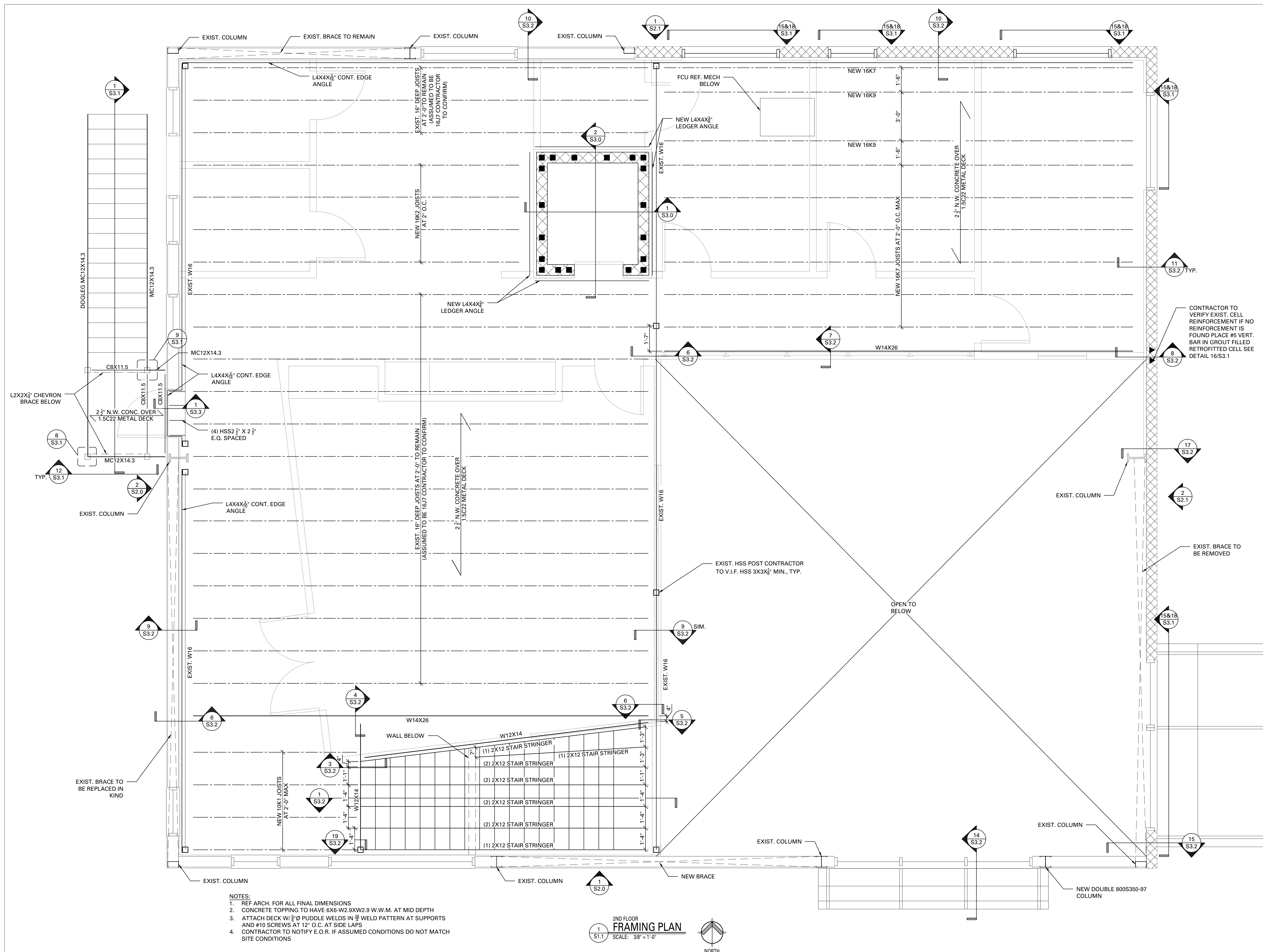
THE WAY A RENOVATION AT:

FOR:

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PROFESSIONAL SEA

S1.1





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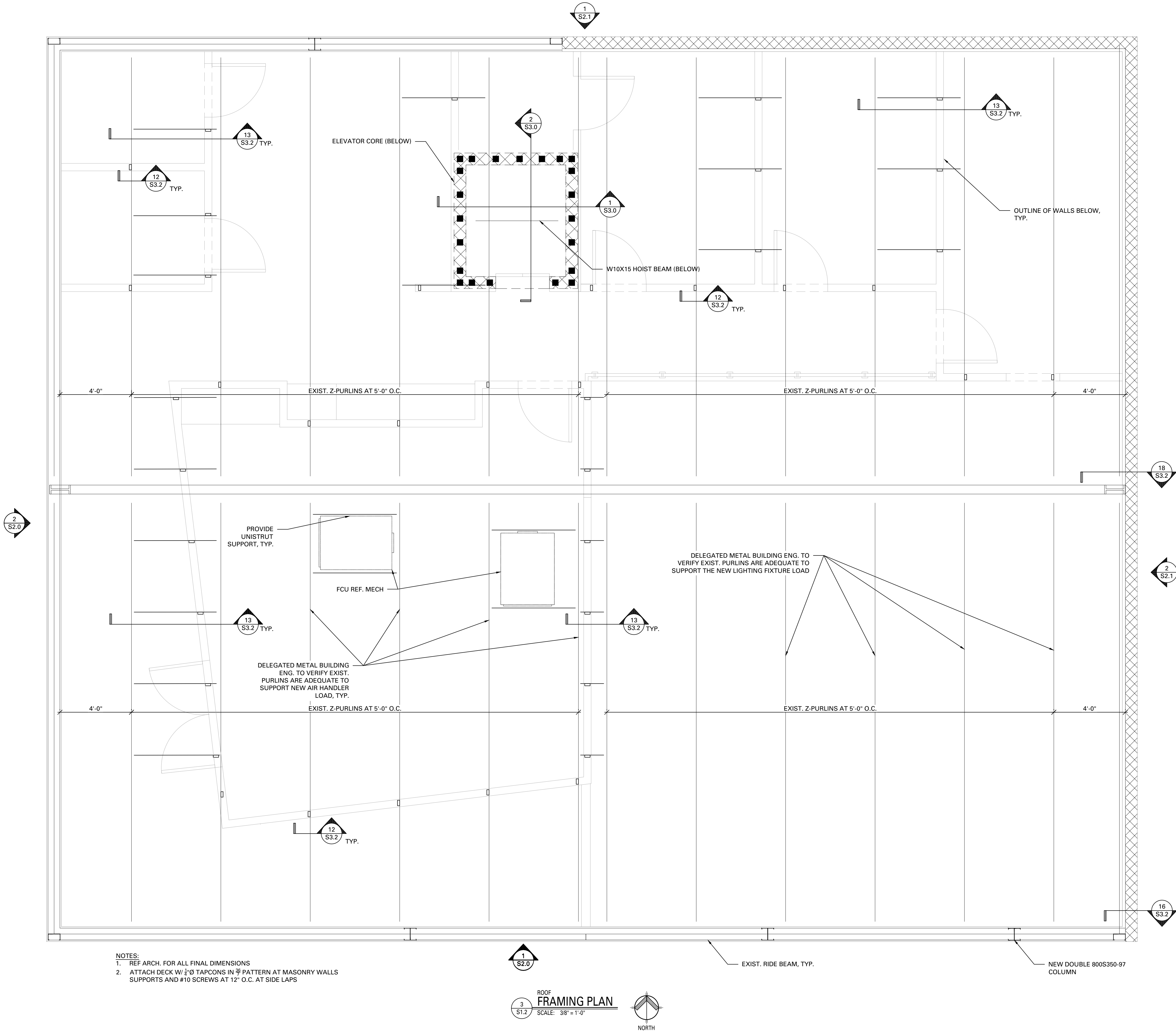
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06	HILLS CTY PPR	06-13-24
07	PERMIT SET	09-25-24
09	BIDDING DOCUMENTS	04-19-25

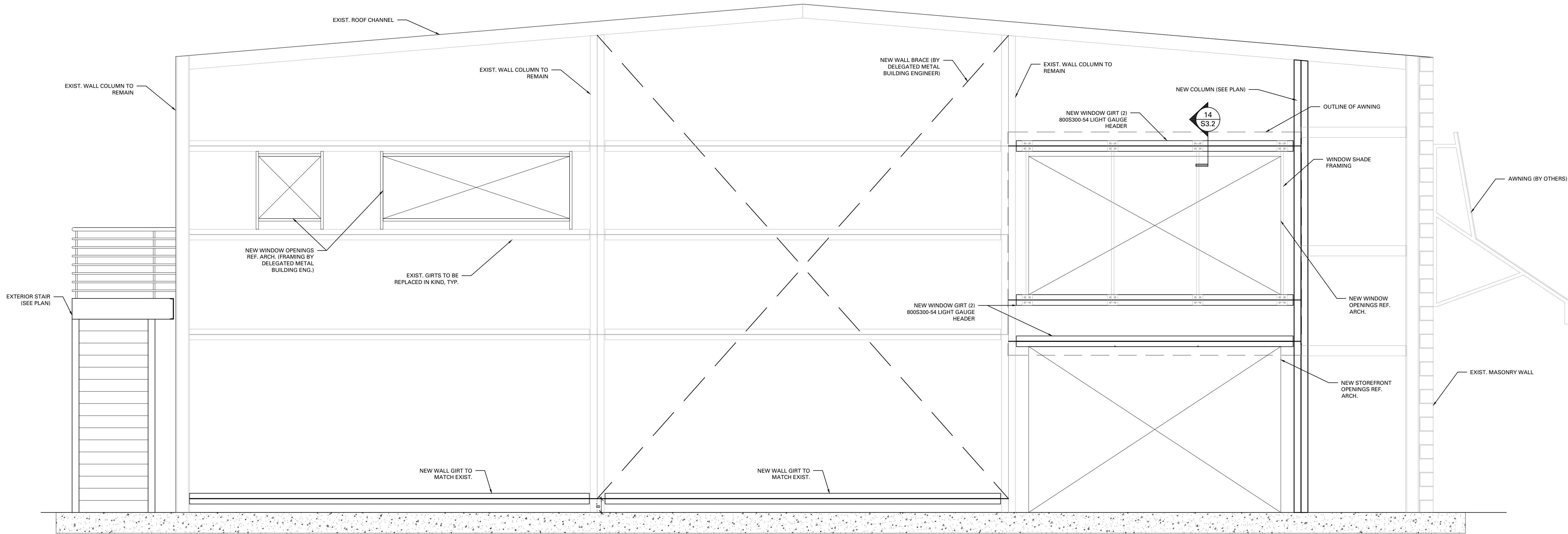
PROFESSIONAL SEAL

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PROJECT NO: 1805

ROOF FRAMING
PLAN

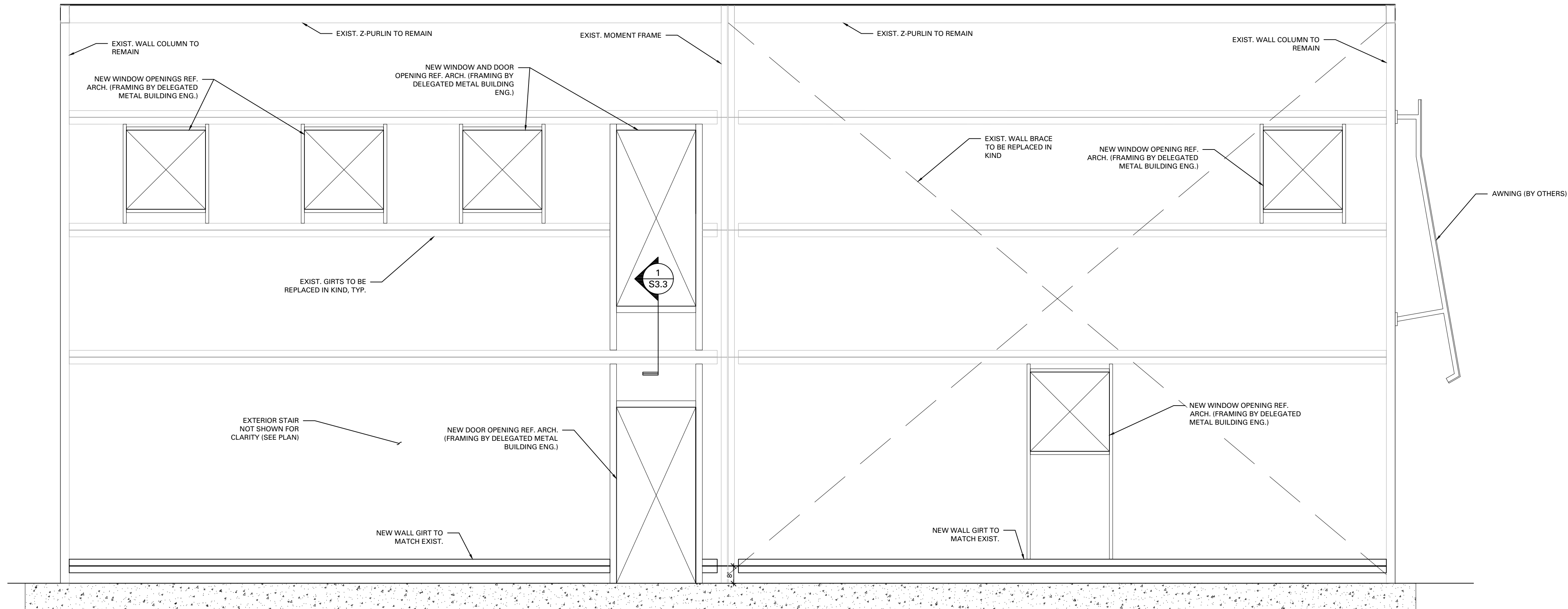
S1.2





1
S2.0
SOUTH FACE
ELEVATION
SCALE: 3/8" = 1'-0"

NOTES:
1. FINISHED EXTERIOR WALL PANELS NOT SHOWN FOR CLARITY



2
S2.0
WEST FACE
ELEVATION
SCALE: 3/8" = 1'-0"

NOTES:
1. FINISHED EXTERIOR WALL PANELS NOT SHOWN FOR CLARITY



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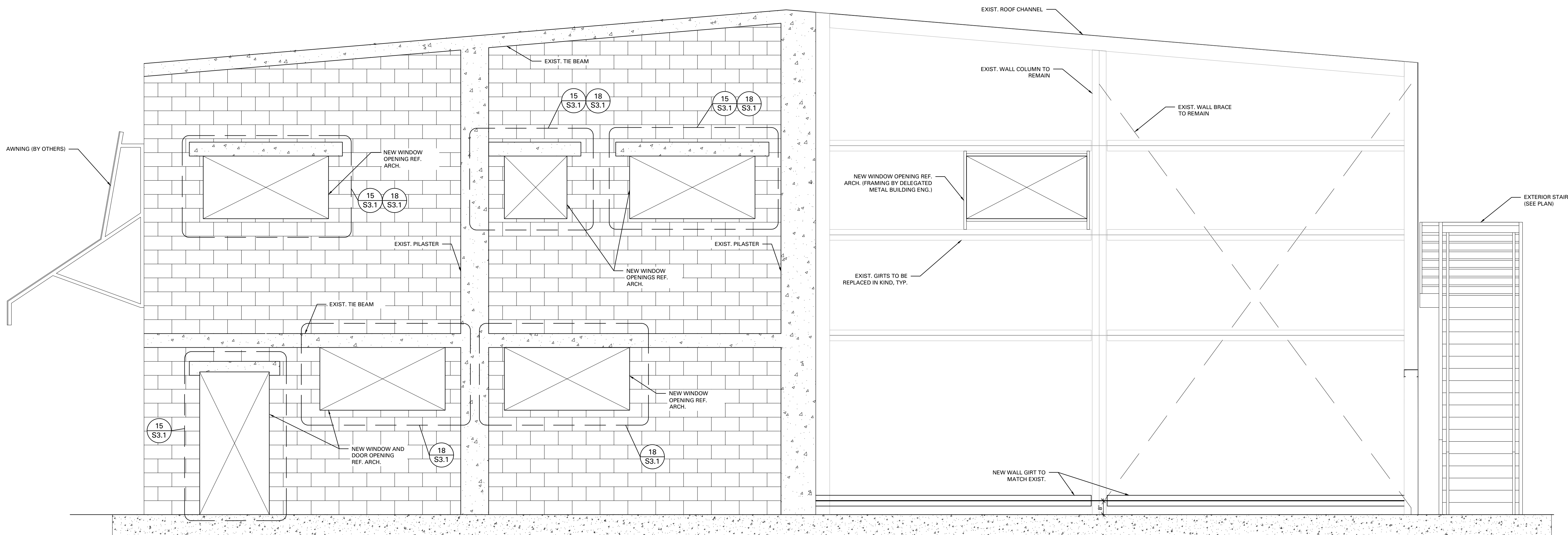
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05	100% CD SET	02-21-24
06	HILLS CTY PPR	06-13-24
07	PERMIT SET	09-25-24
09	BIDDING DOCUMENTS	04-18-25

PROFESSIONAL SEAL

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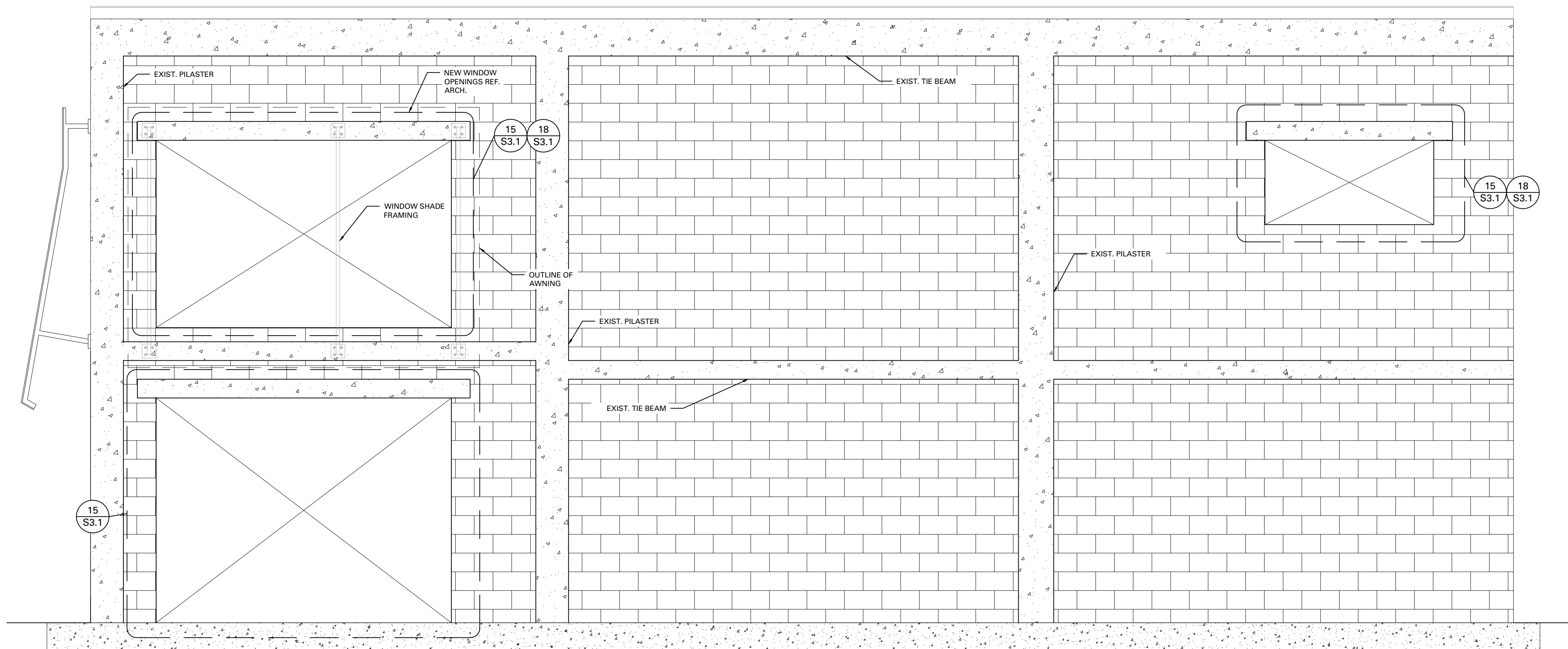
BUILDING SECTION

S2.0



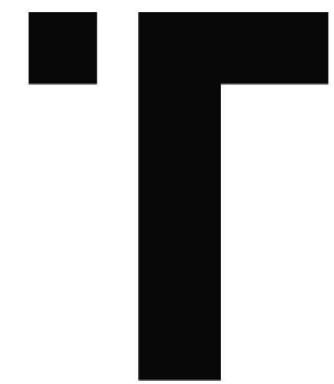
1
S2.1
NORTH FACE
ELEVATION
SCALE: 3/8" = 1'-0"

NOTES:
1. FINISHED EXTERIOR WALL PANELS NOT SHOWN FOR CLARITY



2
S2.1
EAST FACE
ELEVATION
SCALE: 3/8" = 1'-0"

NOTES:
1. FINISHED EXTERIOR WALL PANELS NOT SHOWN FOR CLARITY



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TAMPA, FL 33613

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BUILDING SECTION

S2.1



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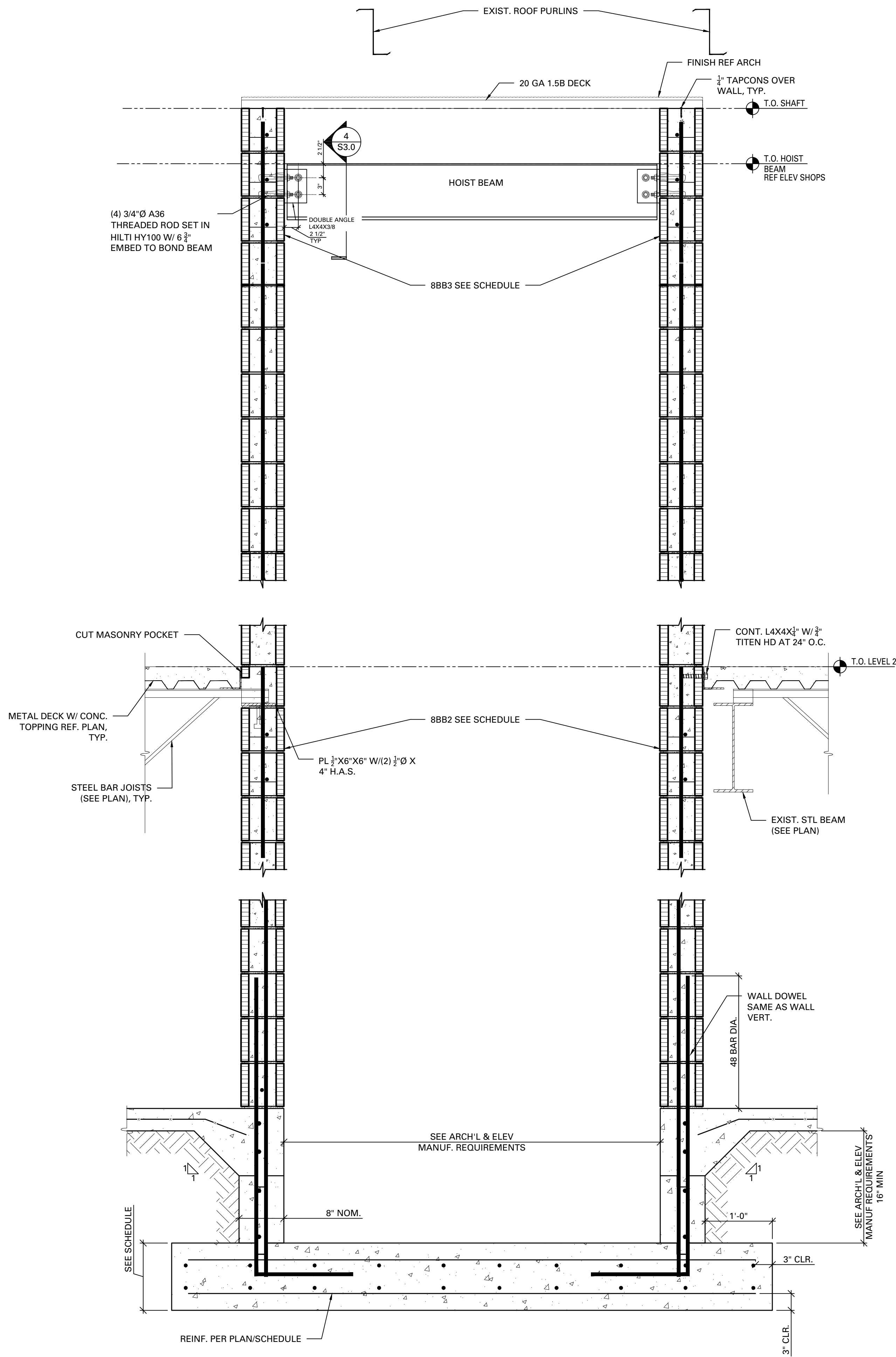
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06	HILLS CTY PPR	06-13-24
07	PERMIT SET	09-25-24
09	BIDDING DOCUMENTS	04-19-25

PROFESSIONAL SEAL

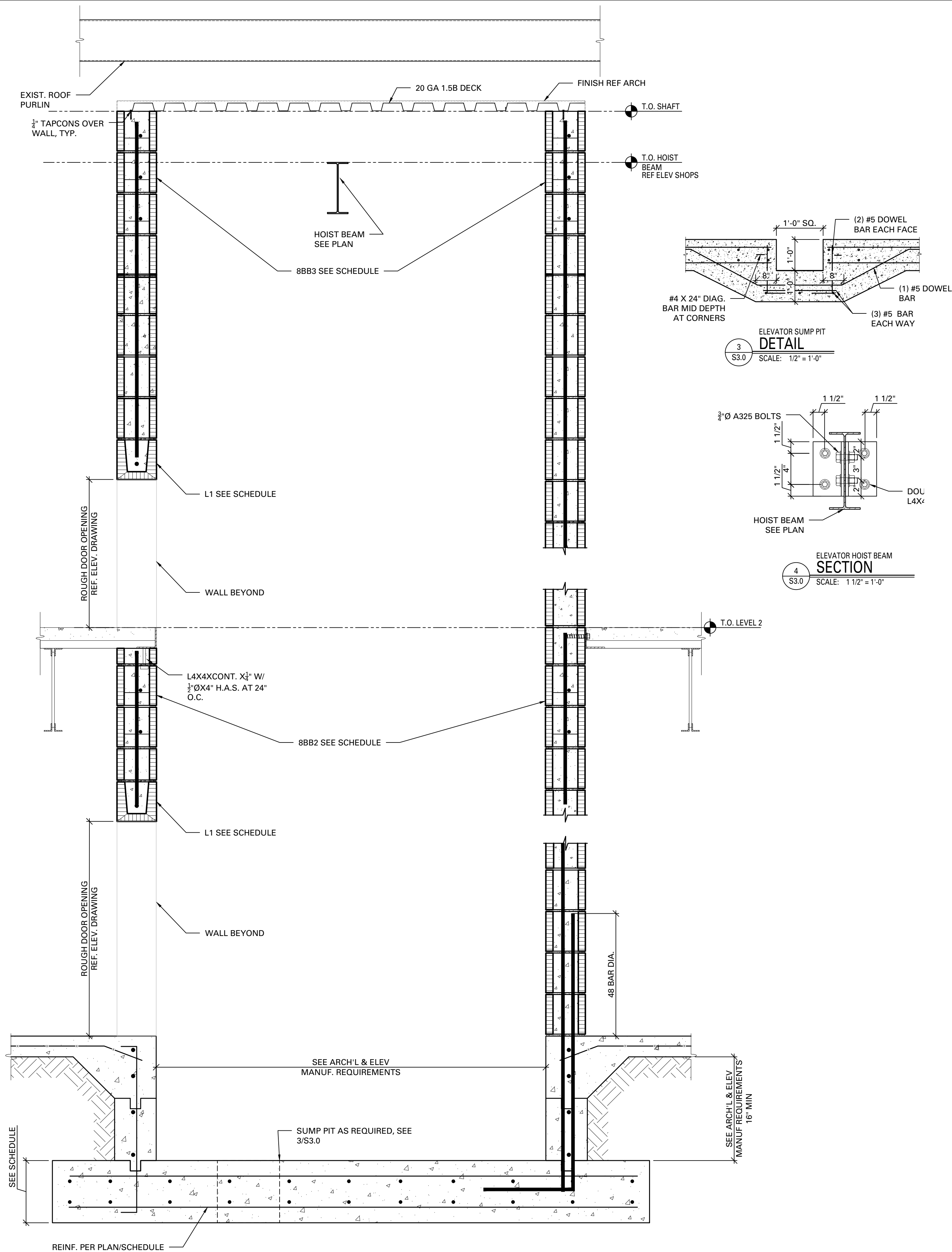
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PROJECT NO: 1805

SECTIONS &
DETAILS

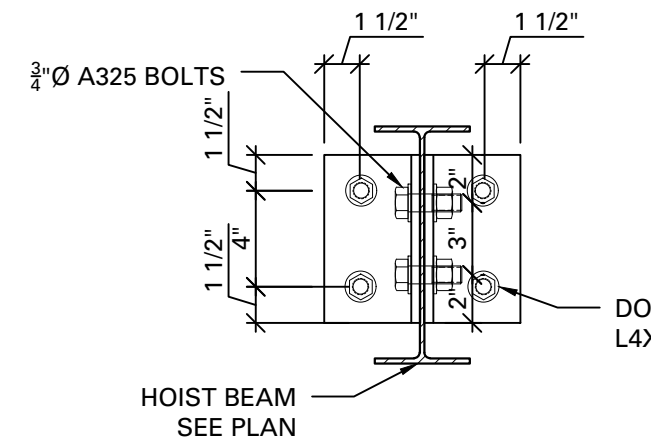
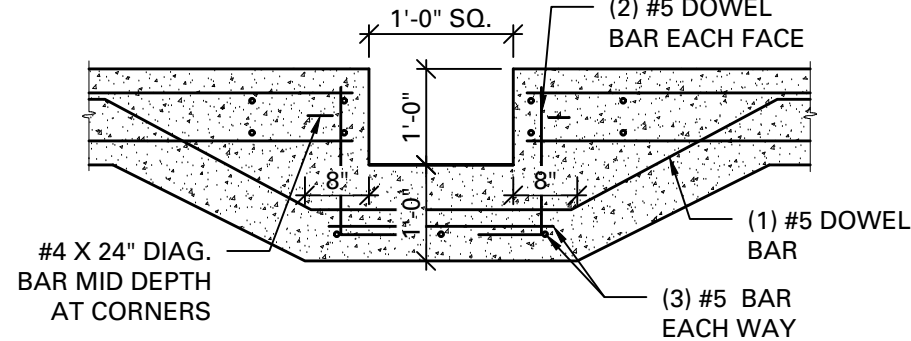
S3.0

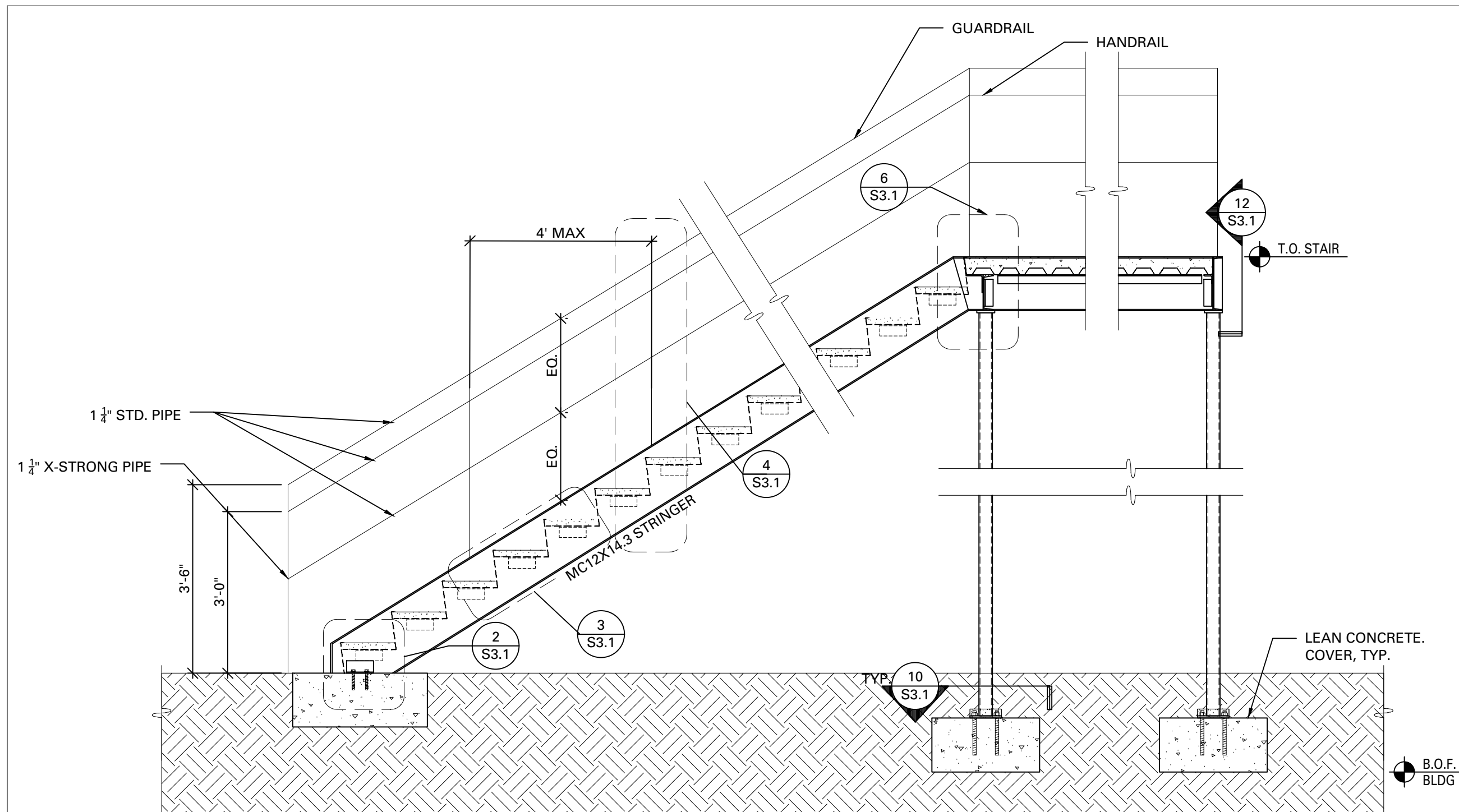


NOTE:
WATERPROOFING AND SUMP PIT IF REQUIRED

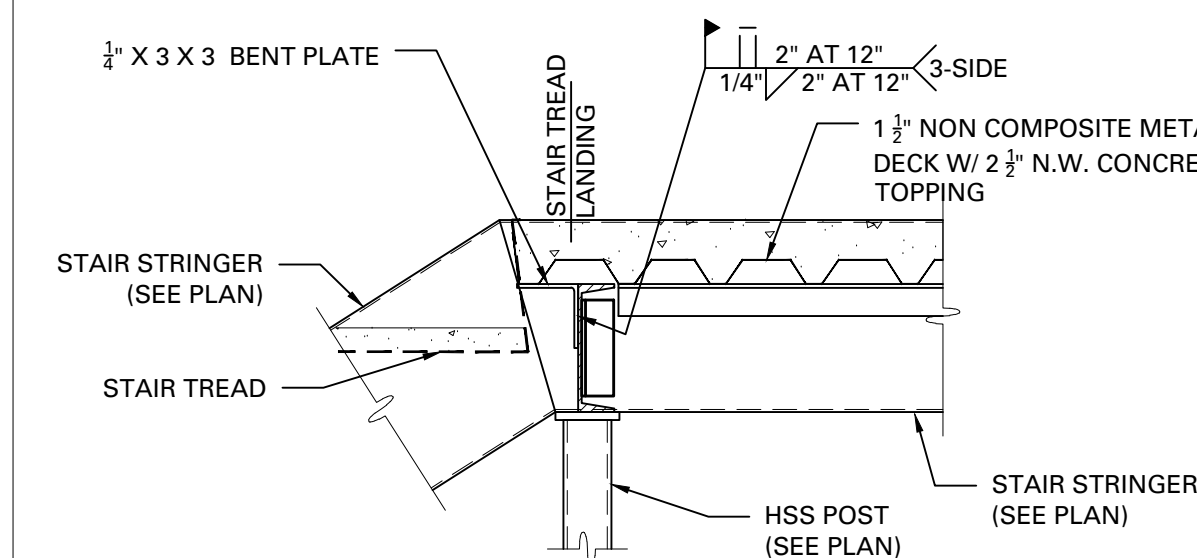


NOTE:
1. WATERPROOFING AND SUMP PIT IF REQUIRED
2. SEE DETAIL 1/S3.0 FOR ITEMS SHOWN BUT NOT LABELED

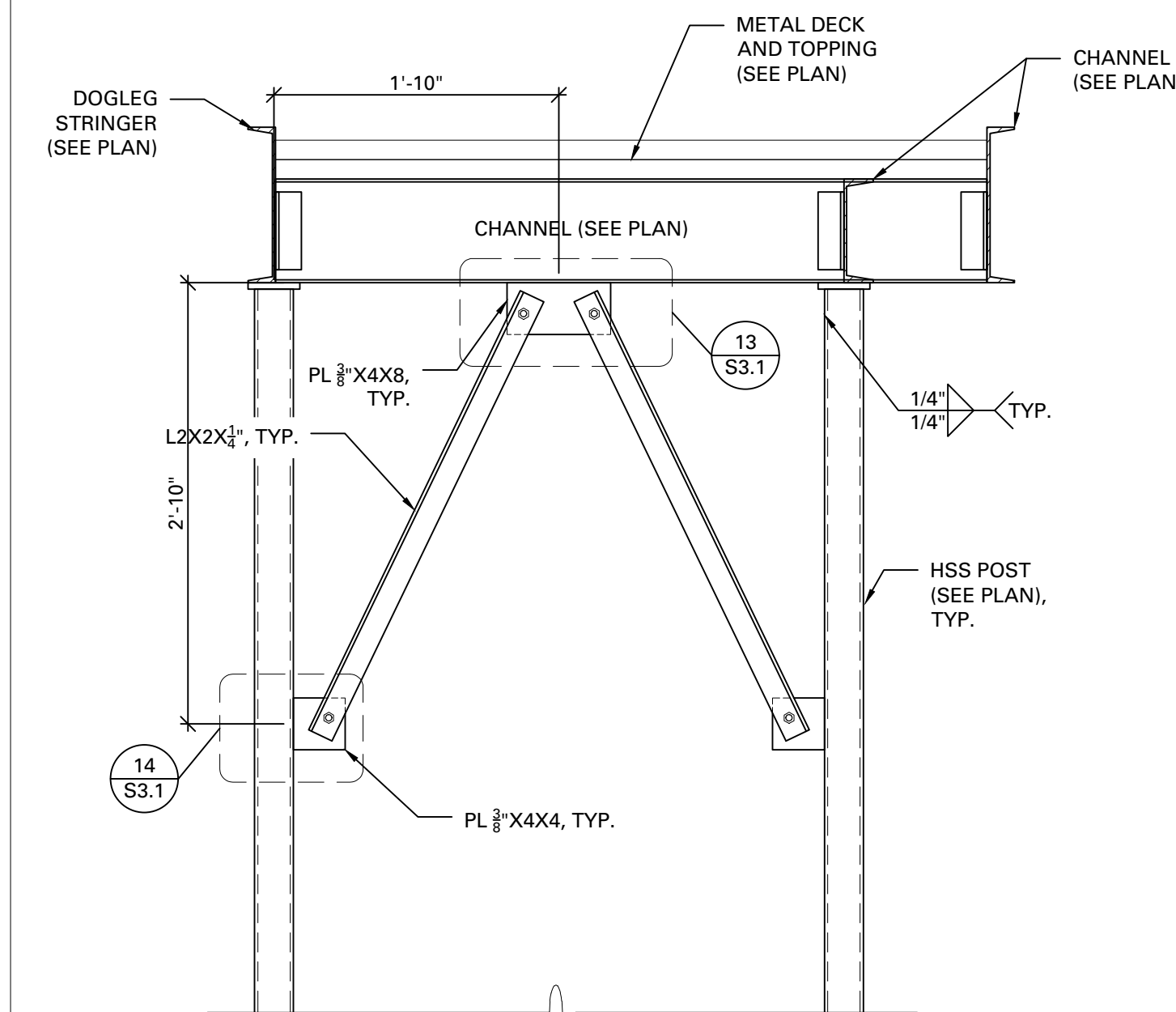




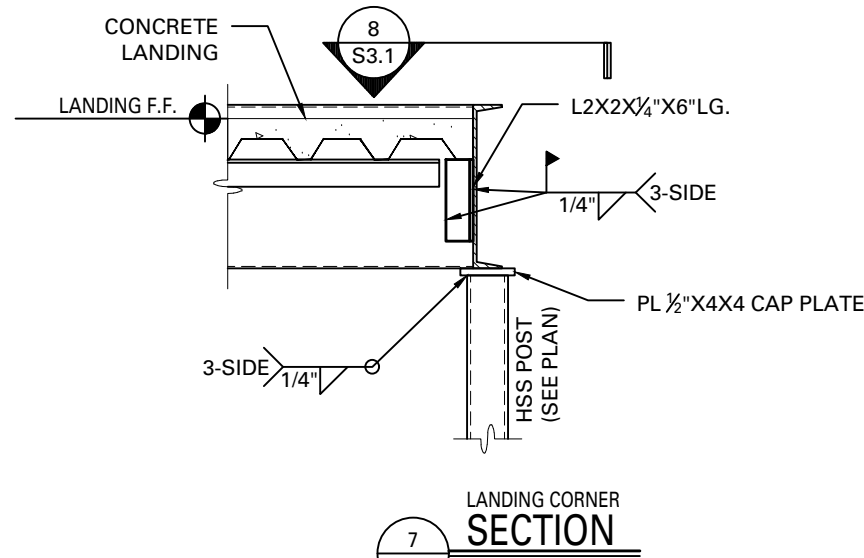
STAIR
SECTION
1
S3.1
SCALE: 1/2" = 1'-0"



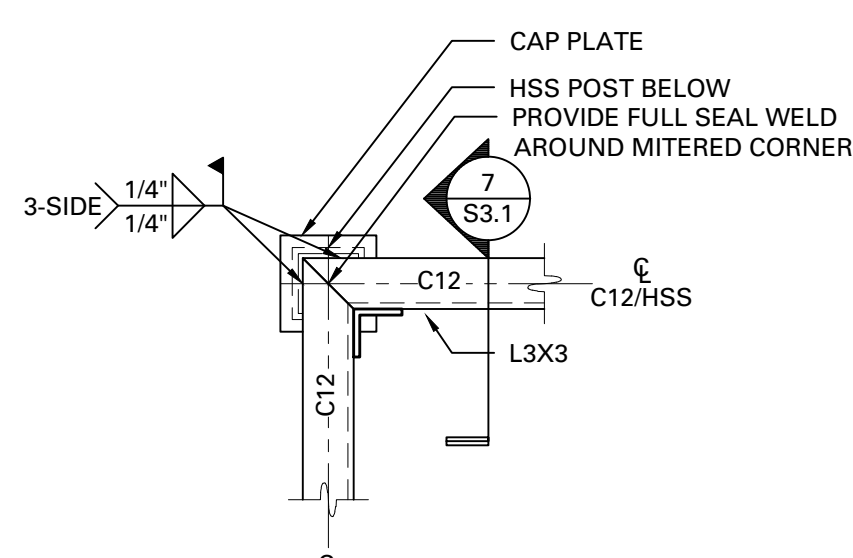
LANDING
SECTION
6
S3.1
SCALE: 1" = 1'-0"



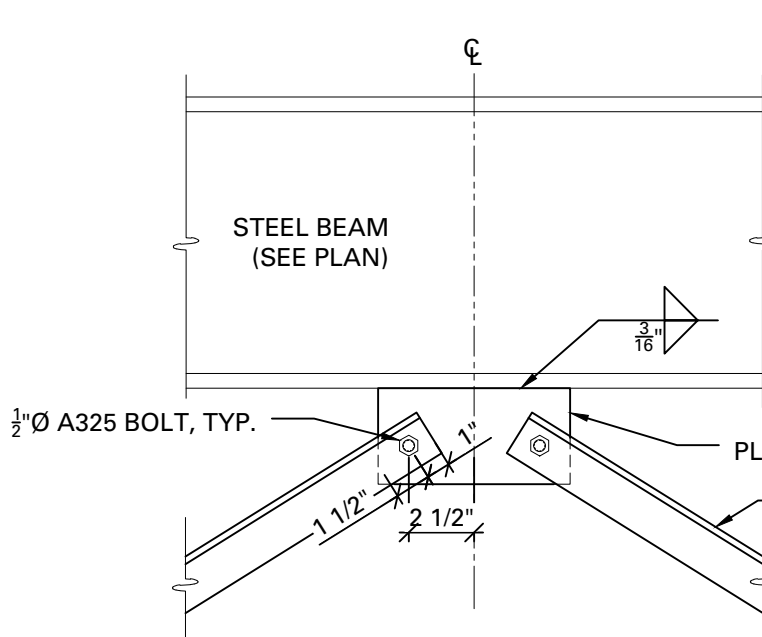
STAIR
SECTION
12
S3.1
SCALE: 1" = 1'-0"



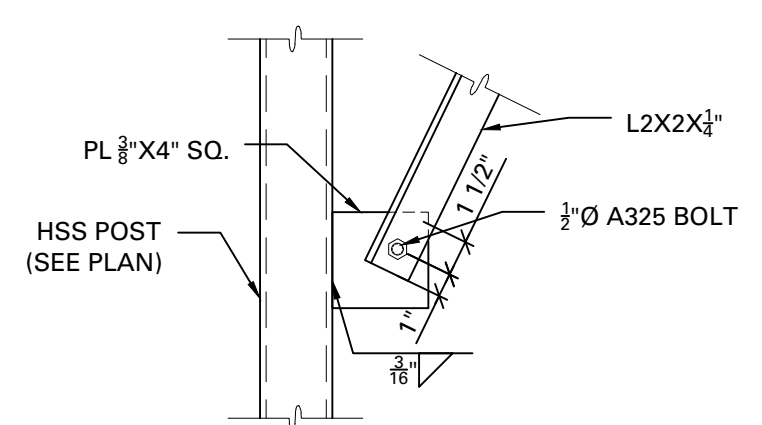
LANDING CORNER
SECTION
7
S3.1
SCALE: 1" = 1'-0"



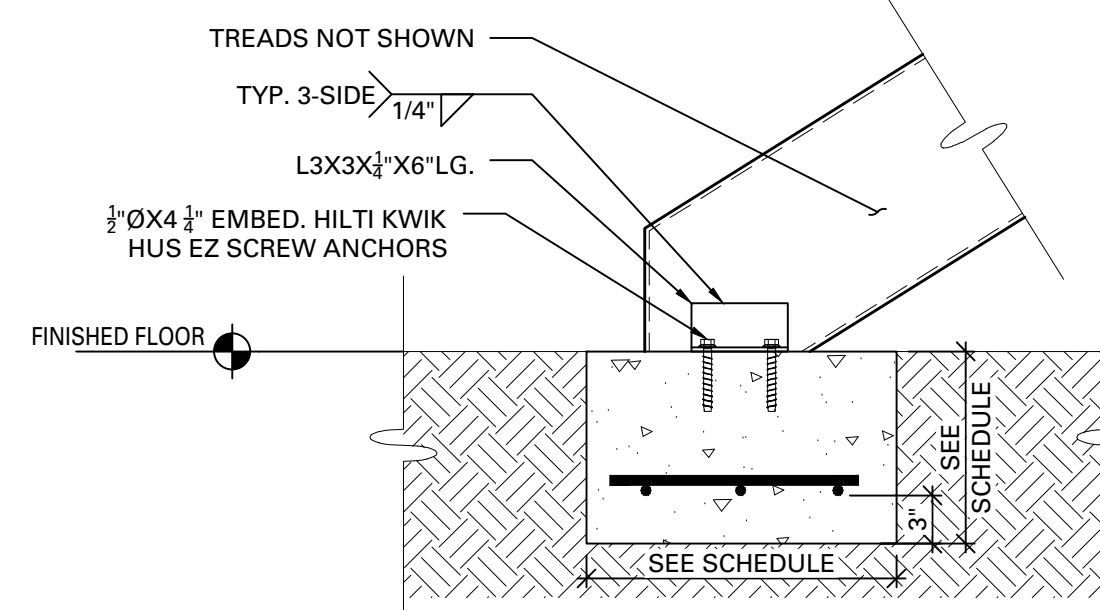
LANDING CORNER
CONN. DETAIL
8
S3.1
SCALE: 1 1/2" = 1'-0"



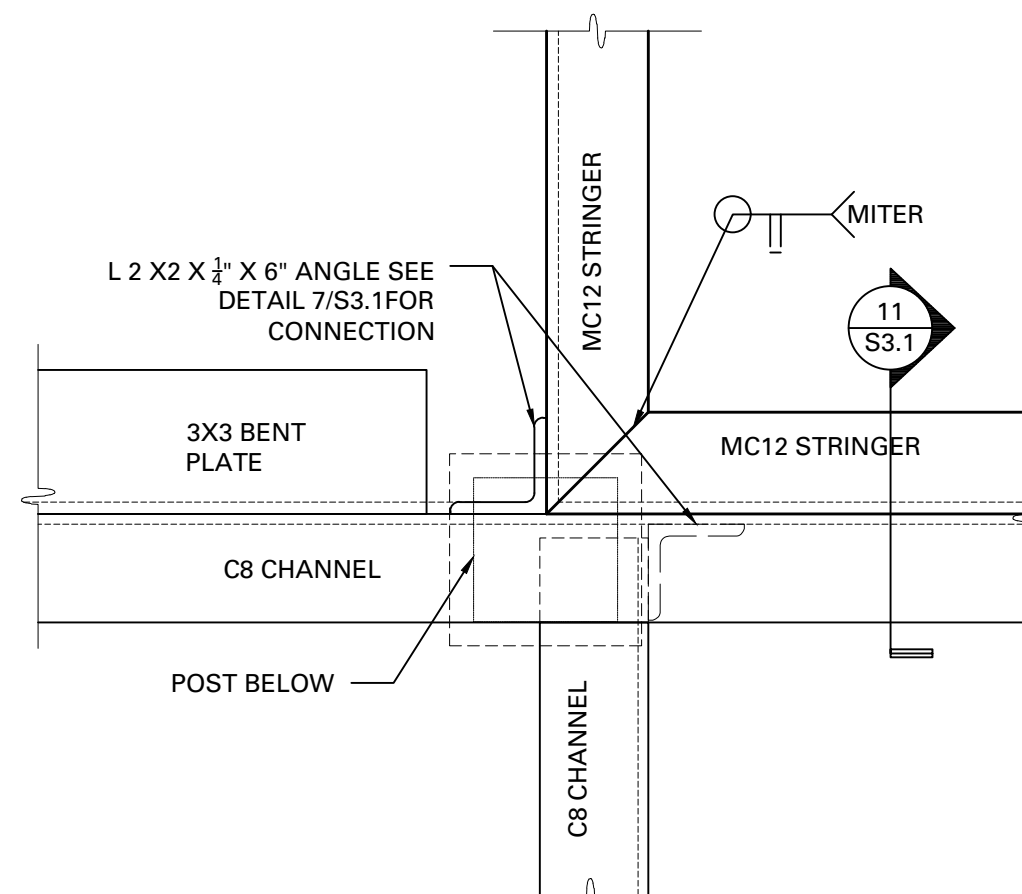
BRACE
CONNECTION DETAIL
13
S3.1
SCALE: 1 1/2" = 1'-0"



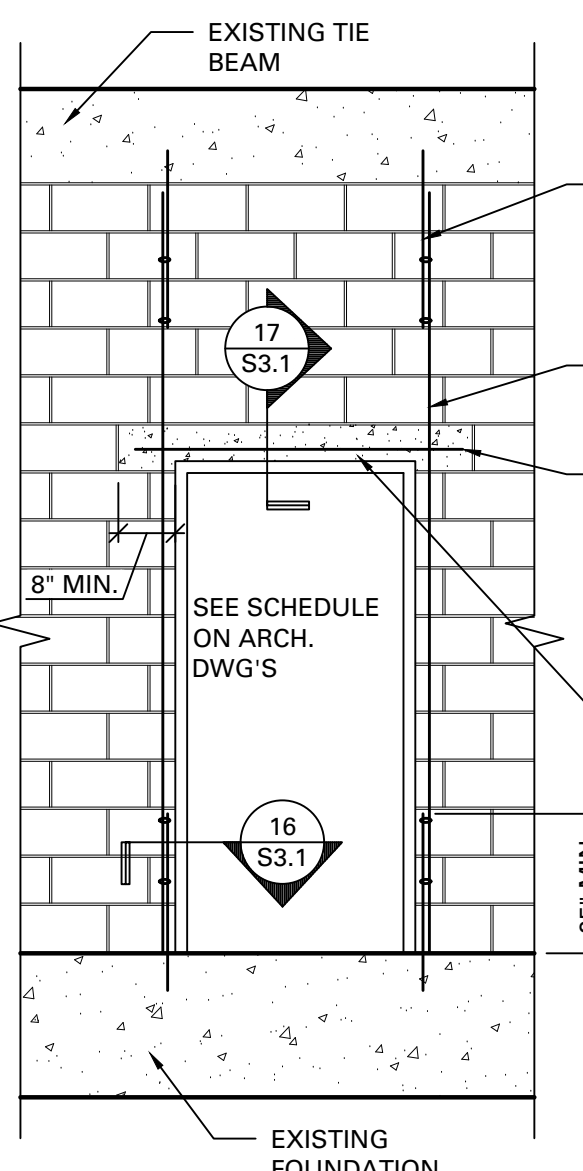
BRACE
CONNECTION DETAIL
14
S3.1
SCALE: 1 1/2" = 1'-0"



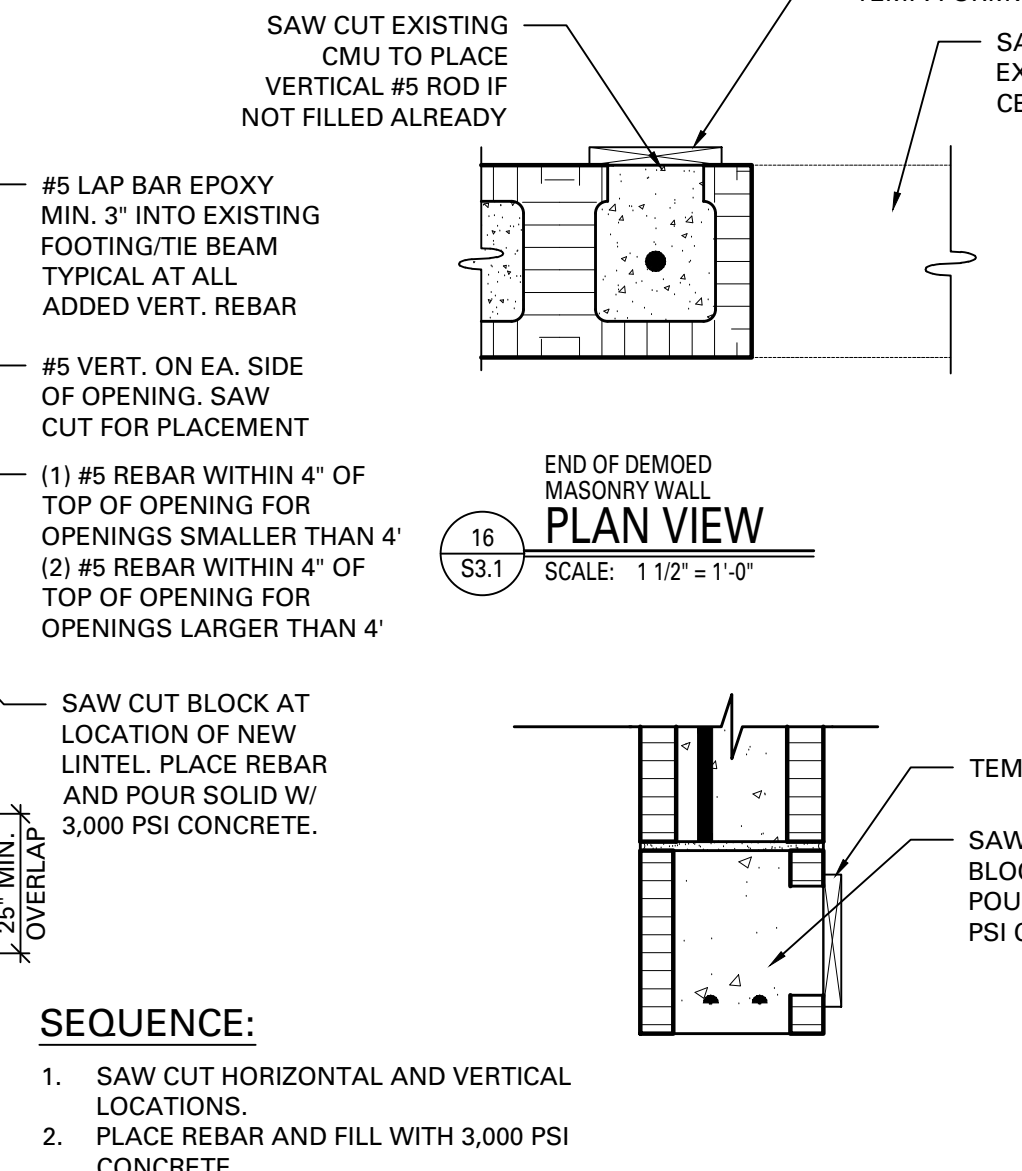
STRINGER BASE
CONNECTION DETAIL
2
S3.1
SCALE: 1" = 1'-0"



LANDING STRINGER
DETAIL
9
S3.1
SCALE: 3" = 1'-0"

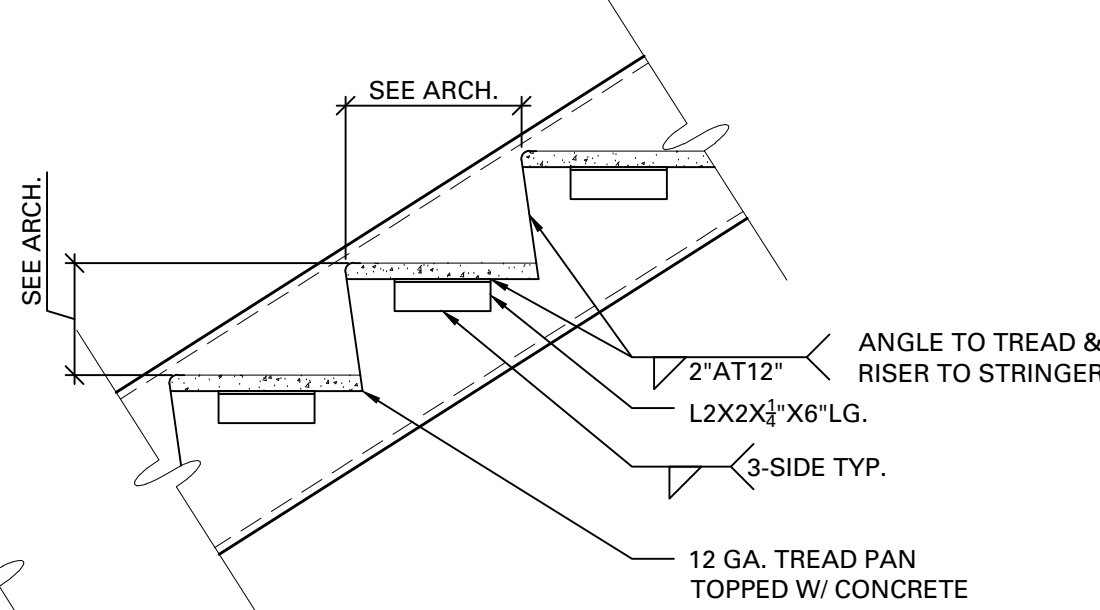


EXISTING OPENING
REINFORCEMENT
ELEVATION
15
S3.1
SCALE: N.T.S.

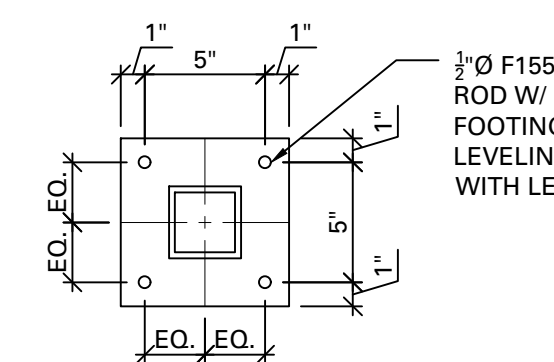


ADDED LINTEL
TO MASONRY WALL
SECTION
17
S3.1
SCALE: 1 1/2" = 1'-0"

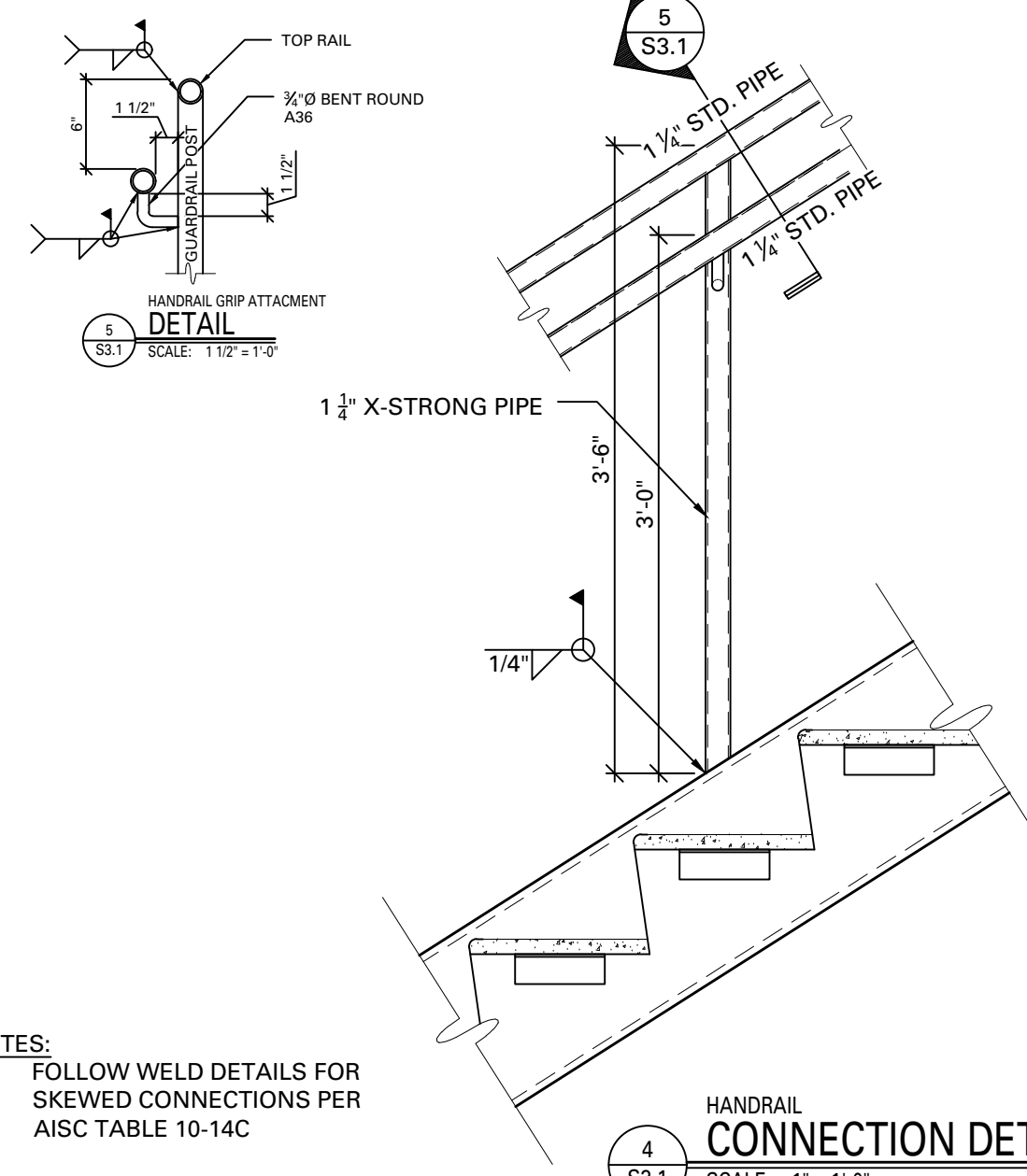
NOTES:
1. ALL REBAR JOINTS SHALL BE OVERLAPPED MIN. 40 TIMES REBAR DIAMETER AND TIE WRAPPED.



STAIR TREAD
CONNECTION DETAIL
3
S3.1
SCALE: 1" = 1'-0"

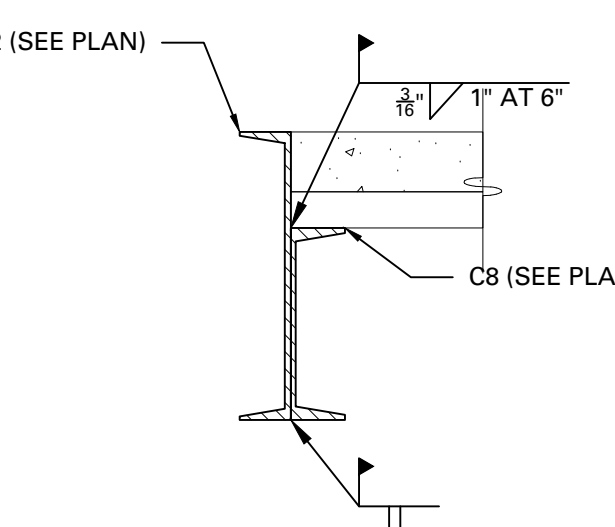


COLUMN BASE PLATE
CONNECTION DETAIL
10
S3.1
SCALE: 1 1/2" = 1'-0"

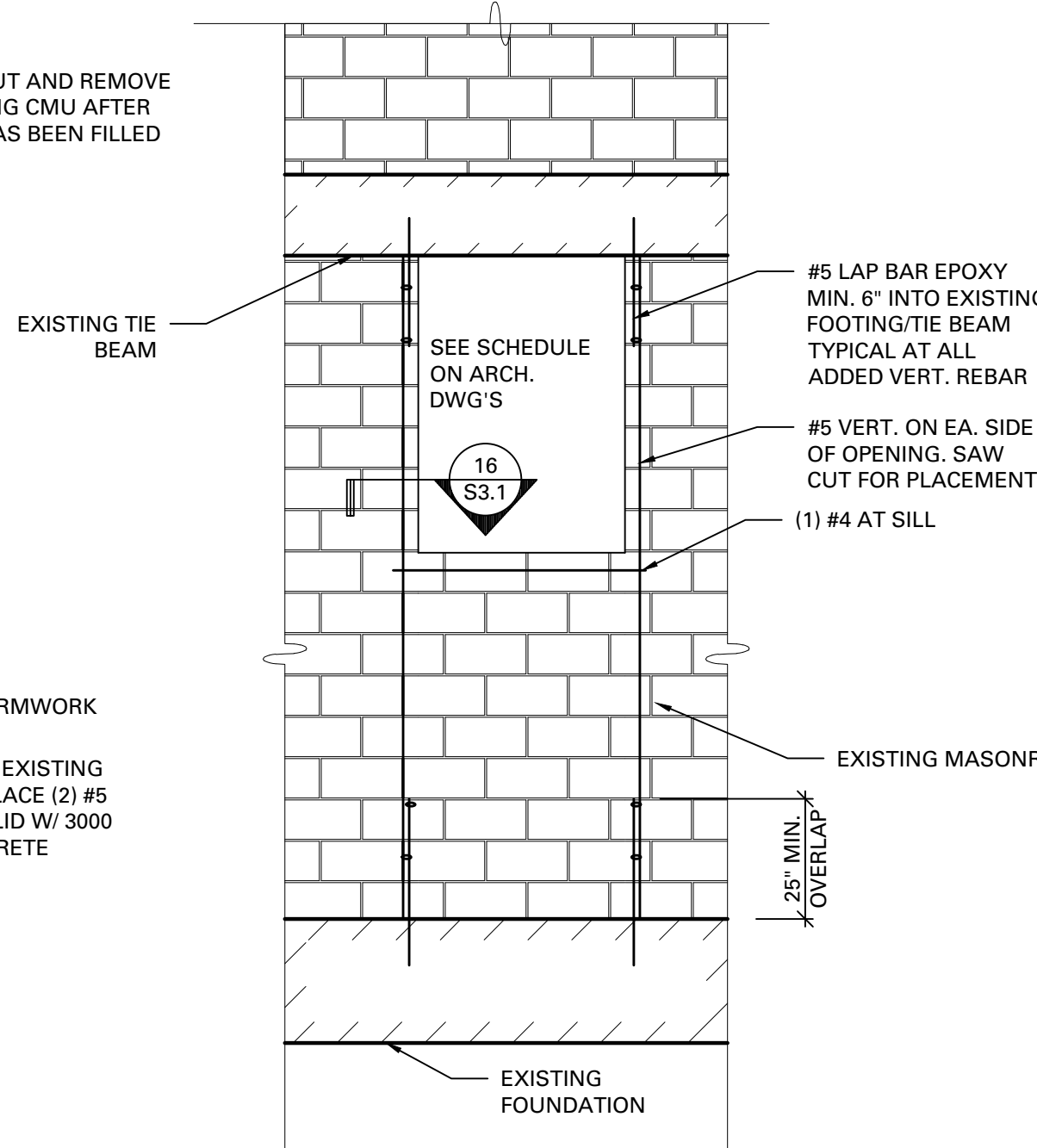


HANDRAIL
CONNECTION DETAIL
4
S3.1
SCALE: 1" = 1'-0"

NOTES:
1. FOLLOW WELD DETAILS FOR SKEWED CONNECTIONS PER AISC TABLE 10-14C



LANDING
CONNECTION DETAIL
11
S3.1
SCALE: 1 1/2" = 1'-0"



NEW OPENING
ELEVATION
18
S3.1
SCALE: N.T.S.

NO.	ISSUE DESCRIPTION	DATE
05	100% CD SET	03-21-24
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07	PERMIT SET	09-25-24
09	BIDDING DOCUMENTS	04-19-25

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SECTIONS & DETAILS



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THE WAREHOUSE

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TAMPA, FL 33613

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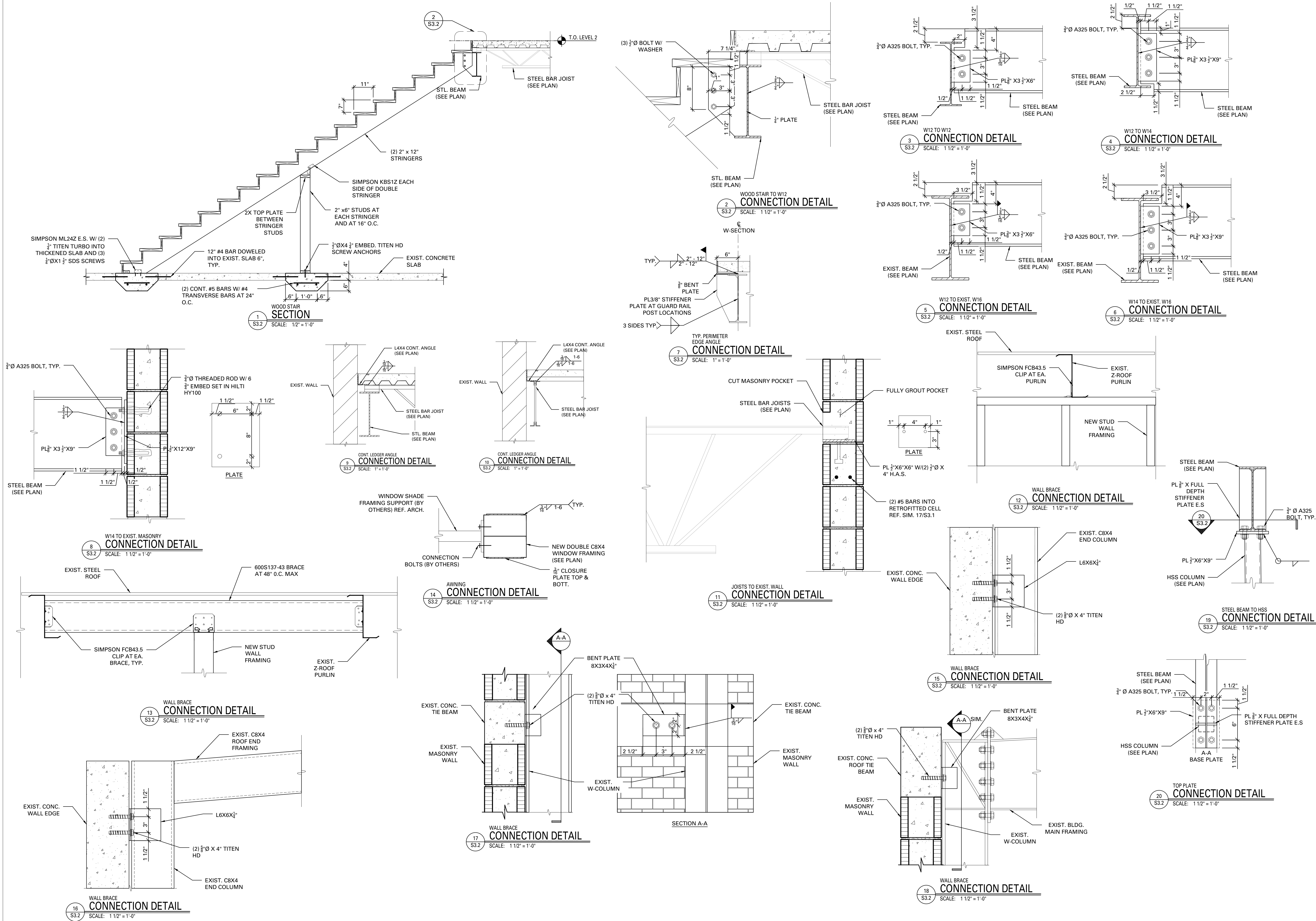
NO.	ISSUE DESCRIPTION	DATE
05	100% CD SET	03-21-24
06	HILLS CTRY PPR	06-13-24
07	PERMIT SET	09-25-24
09	BIDDING DOCUMENTS	04-19-25
10		
11		
12		
13		
14		
15		
16		
17		
18		
19		
20		

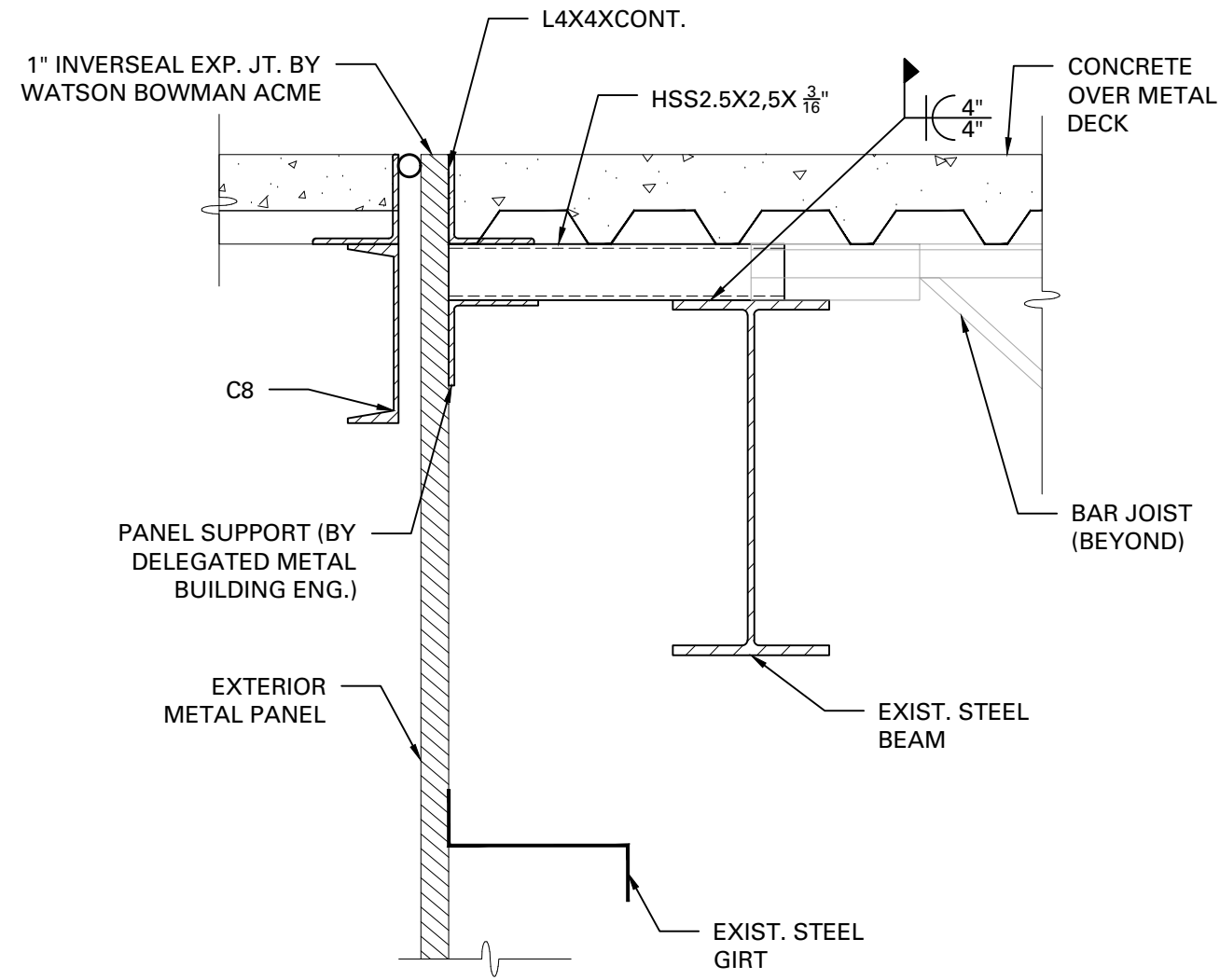
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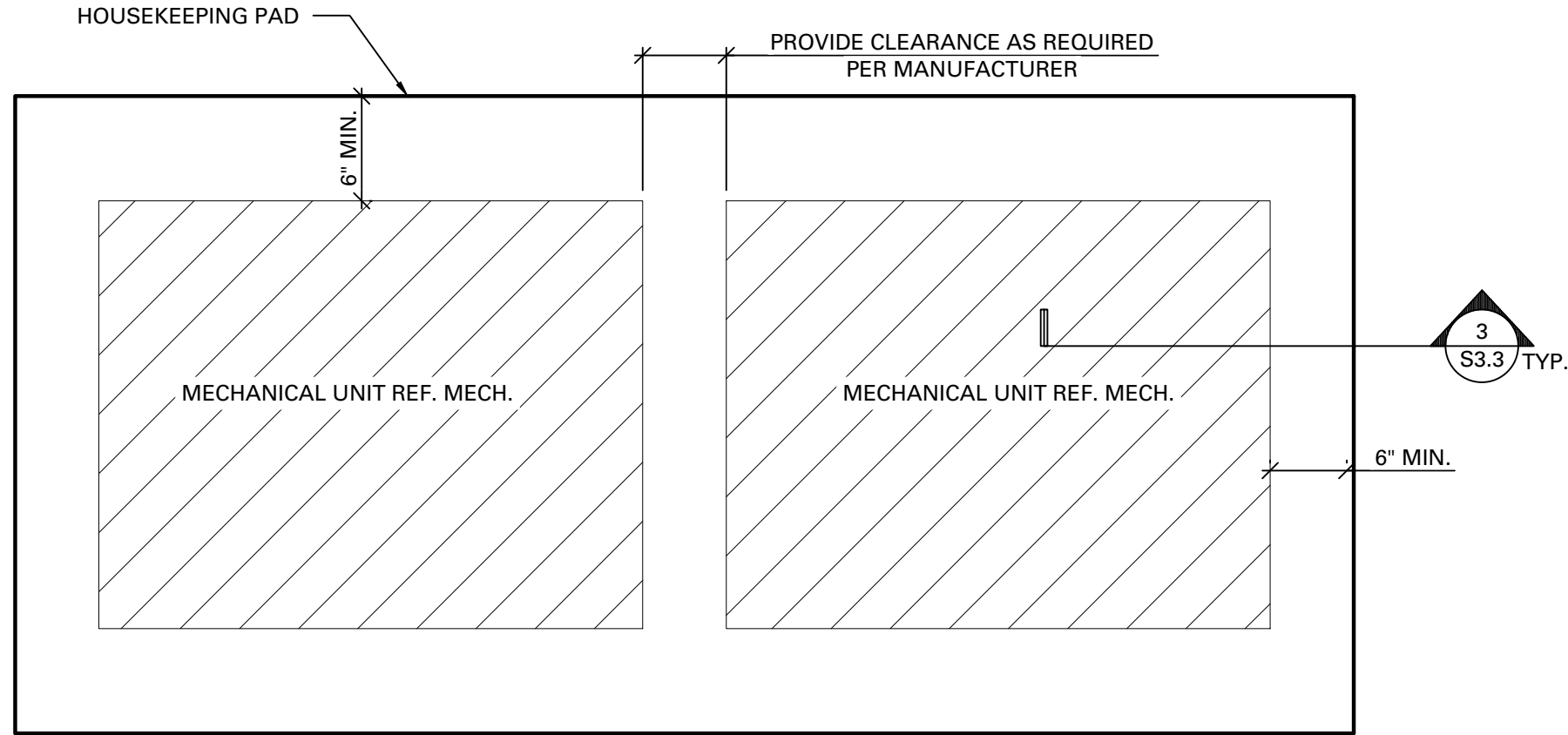
SECTIONS &
DETAILS

S3.2



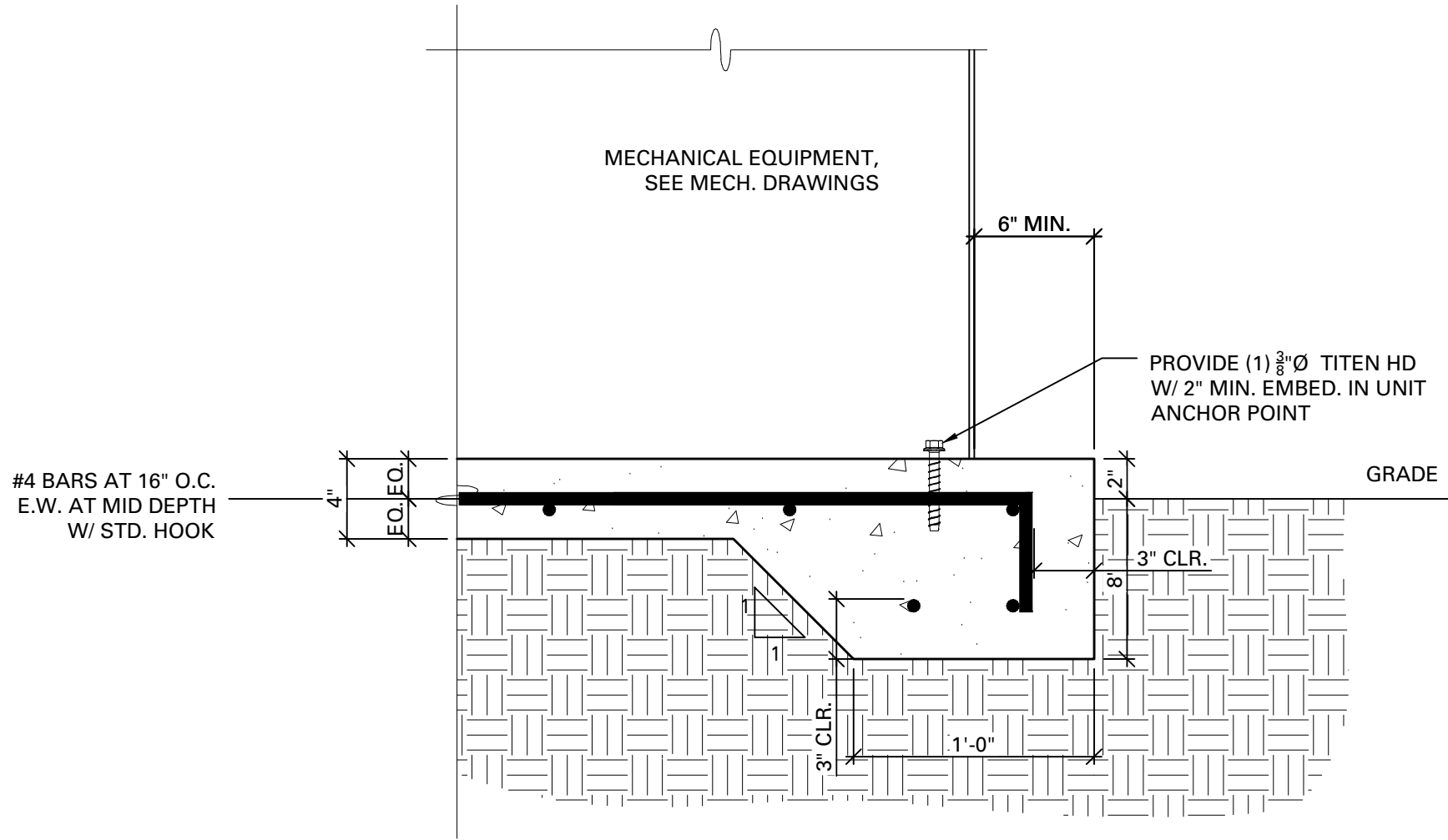


1
S3.3
SECTION
SCALE: 1 1/2" = 1'-0"



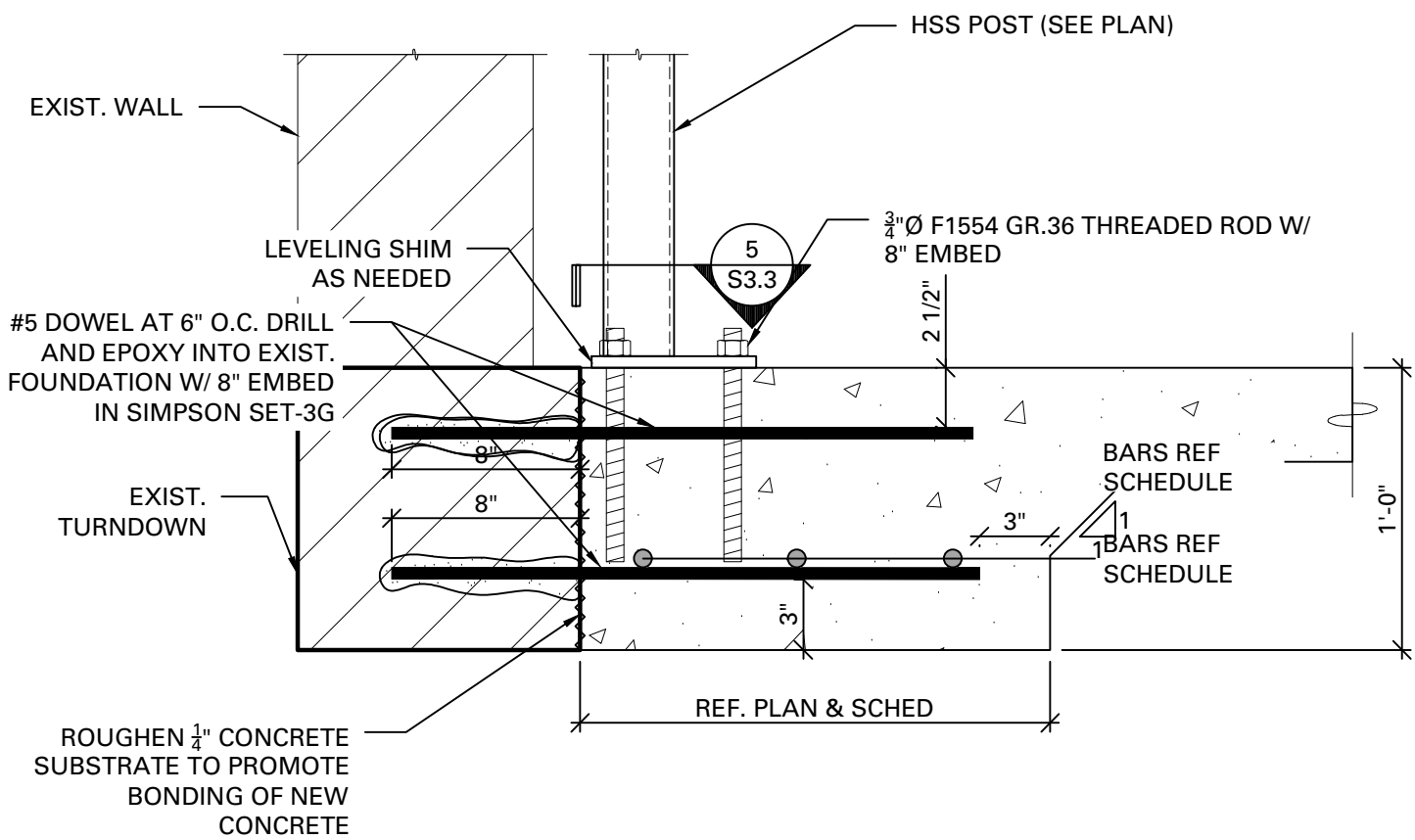
2
S3.3
TYPICAL CONCRETE
HOUSE KEEPING PAD
SCHEMATIC
SCALE: 1" = 1'-0"
NORTH

- NOTE:
- PAD SIZE TO BE VERIFIED WITH MANUFACTURER'S REQUIREMENTS. PAD TO BE MINIMUM 0'-6" LARGER IN BOTH DIRECTIONS THAN UNIT. CONTRACTOR MAY INCREASE SIZE AS REQUIRED.
 - REF. FOUNDATION PLAN AND MECHANICAL DRAWINGS FOR FINAL LOCATION OF PAD.

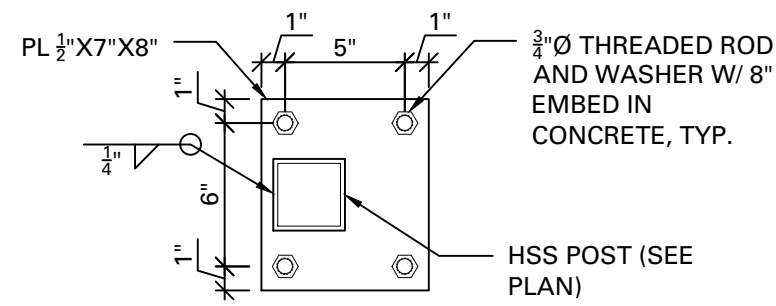


3
S3.3
HOUSEKEEPING PAD
SECTION
SCALE: 1 1/2" = 1'-0"

- NOTES:
- MECHANICAL EQUIPMENT CLEARANCE TO BE IN ACCORDANCE WITH MANUFACTURERS MINIMUM REQUIREMENTS.
 - SLAB TO BE CENTERED UNDER UNIT .
 - SLAB TO EXTEND MINIMUM OF 6" BEYOND EACH EDGE.
 - VERIFY SLAB EDGE DISTANCE WITH EQUIPMENT SUPPLIER AND LOCAL BUILDING ORDINANCE.



4
S3.3
PAD FOOTING
FOUNDATION DETAIL
SCALE: 1 1/2" = 1'-0"



5
S3.3
BASE PLATE
CONNECTION DETAIL
SCALE: 1 1/2" = 1'-0"

- NOTE:
- PROVIDE LEVELING SHIM AS NEEDED



TRACTION

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WHEN SIGNED & SEALED TO THE BEST OF THE
ARCHITECT'S KNOWLEDGE, THESE PLANS COMPLY W/
THE MINIMUM REQUIREMENTS OF THE APPLICABLE
BUILDING CODES.

THE WAREHOUSE

A RENOVATION AT:

1916 E 136 AVE
TAMPA, FL 33613

FOR:

UNIVERSITY AREA COMMUNITY
DEVELOPMENT CORP (UACDC)

NO.	ISSUE DESCRIPTION	DATE
05	100% CD SET	02-21-24
06	HILLS CTY PPR	06-13-24
07	PERMIT SET	09-25-24
09	BIDDING DOCUMENTS	04-18-25

PROFESSIONAL SEAL

DRAWN: DS
REVIEWED: TL
PROJECT NO: 1805

SECTIONS &
DETAILS

S3.3